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TRAFFIC IMPACT STUDY

PROPOSED RESIDENTIAL DEVELOPMENT

HAWK'S CREEK

BROWNSBURG, INDIANA

PREPARED FOR

David Weekley Homes

A horizontal brushstroke graphic in shades of orange and red, positioned below the company name.

JULY 2024

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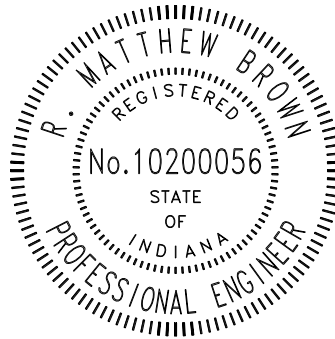
CERTIFICATION

I certify that this **TRAFFIC IMPACT STUDY** has been prepared by me and under my immediate supervision and that I have experience and training in the field of traffic and transportation engineering.

A&F ENGINEERING Co., LLC



July 23, 2024
R. Matt Brown, P.E.
Indiana Registration 10200056



Sri Gayatri Kesara

Sri Gayatri Kesara
Traffic Engineer Technician

INTRODUCTION

This **TRAFFIC IMPACT STUDY**, prepared on behalf of David Weekley Homes, is for a proposed residential development known as “Hawk’s Creek” that will be located south of CR 800 North & west of SR 267 (Green Street) in Brownsburg, Indiana.

PURPOSE

The purpose of this analysis is to determine what impact traffic generated by the proposed residential development will have on the existing adjacent roadway system. This analysis will identify any existing roadway deficiencies or ones that may occur in the future.

Conclusions will be reached that will determine if the roadway system can accommodate the anticipated traffic volumes or will determine the modifications that will be required to the system if there are identified deficiencies.

Recommendations will be made that will address the conclusions resulting from this analysis. These recommendations will address feasible roadway system improvements to provide safe ingress and egress, to and from the proposed residential development, with minimal interference to traffic on the public street system.

SCOPE OF WORK

The scope of work for this analysis is as follows:

First, obtain peak hour turning movement traffic volume counts between the hours of 6:30 AM and 9:00 AM and 3:30 PM and 7:00 PM at the following intersections:

- CR 800 N & CR 650 E
- CR 800 N & SR 267 (Green Street)
- CR 700 N & CR 650 E
- CR 700 N & SR 267 (Green Street)
- SR 267 (Green Street) & Acre Lane

Second, estimate the year 2029 background traffic volumes by applying growth rate factors provided by Shrewsberry to the existing traffic volumes.

Third, estimate the number of peak hour trips that will be generated by the proposed residential development.

Fourth, assign and distribute the generated peak hour traffic from the proposed residential development to each of the study intersections.

Fifth, prepare a capacity analysis, level of service analysis and turn lane analysis at the study intersections for each of the following scenarios:

Scenario 1: Existing Traffic Volumes – Based on existing peak hour traffic volumes.

Scenario 2: Year 2029 Background Traffic Volumes – Based on applying growth rate factors provided by Shrewsbury to the existing traffic volumes.

Scenario 3: Year 2029 + Proposed Development Traffic Volumes – Based on the sum of year 2029 background traffic volumes and generated traffic volumes from the proposed development.

Sixth, prepare recommendations for the roadway geometrics that will be needed to accommodate the future traffic volumes.

Finally, prepare a **TRAFFIC IMPACT STUDY** report documenting all data, analyses, conclusions, and recommendations to provide for the safe and efficient movement of traffic through the study area.

DESCRIPTION OF THE PROPOSED DEVELOPMENT

The proposed development will be located south of CR 800 North & west of SR 267 (Green Street) in Brownsburg, Indiana. The proposed residential development will consist of two parcels. The west parcel will have 89 homes and the east parcel will have 25 homes. As proposed, the west parcel will be served by a full access drive along CR 800 N and a connection to the existing Windridge Lane. The east parcel will be served via a realigned access drive at the existing Connection Pointe Christian Church access location along SR 267 (Green Street). **Figure 1** is an area map showing the location and general layout of the proposed site.

STUDY AREA

The study area for this analysis has been defined to include the following intersections:

- CR 800 N & CR 650 E
- CR 800 N & SR 267 (Green Street)
- CR 700 N & CR 650 E
- CR 700 N & SR 267 (Green Street)
- SR 267 (Green Street) & Acre Lane/
Connection Pointe Christian Church/
Proposed Development Access Drive
- CR 800 N & Proposed Access Drive

Figures 2A & 2B show the existing intersection geometrics at the study intersections.

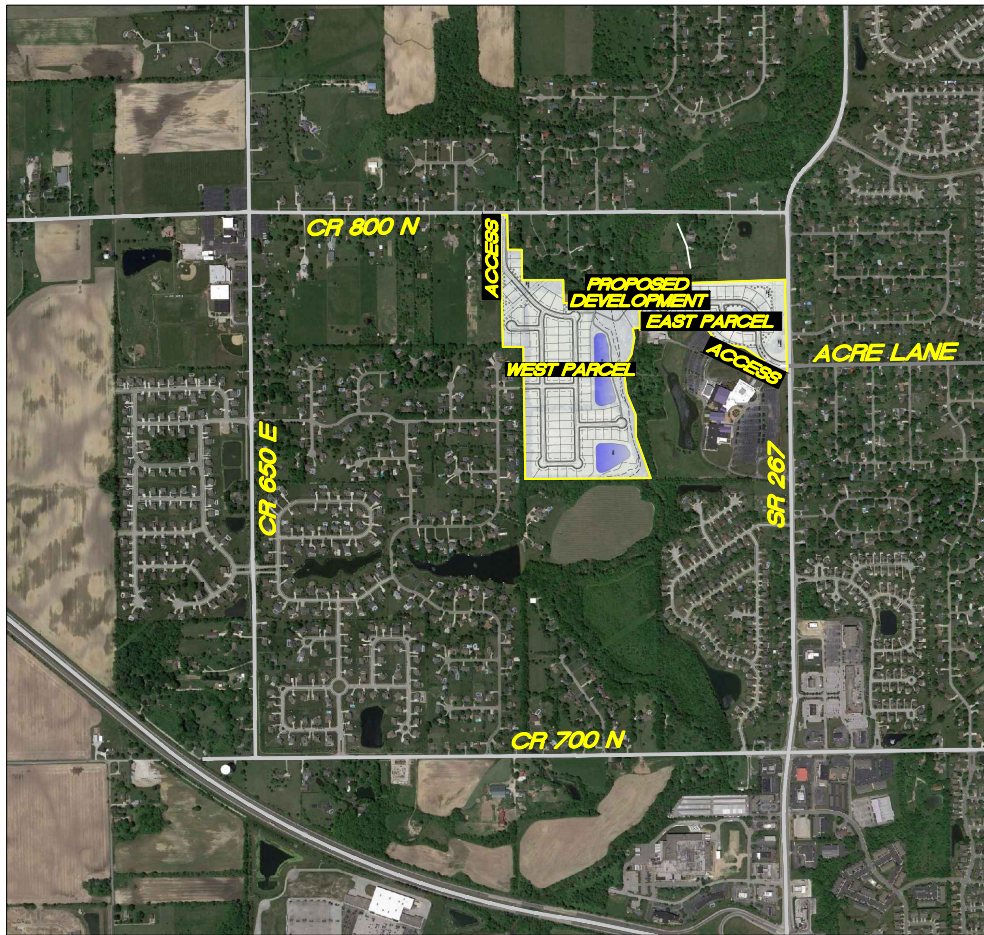
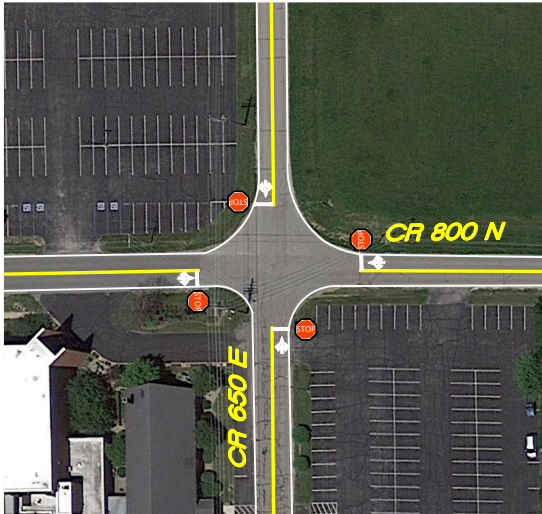
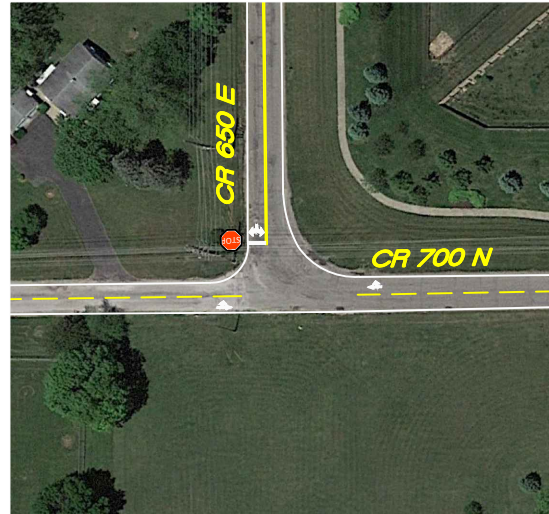


FIGURE 1
AREA MAP

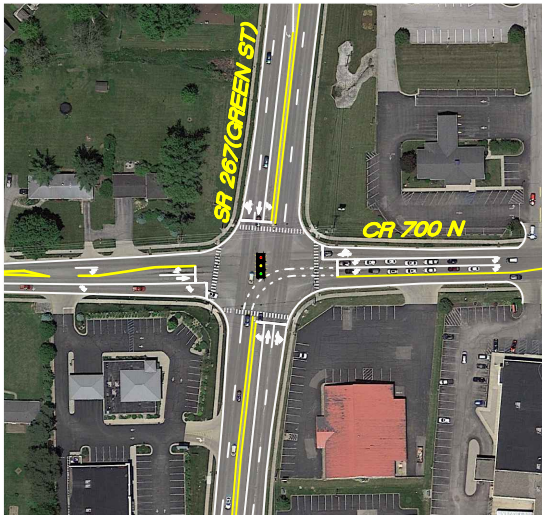
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BROWNSBURG, IN



CR 800 N & CR 650 E



CR 700 N & CR 650 E



CR 700 N & SR 267 (GREEN ST)

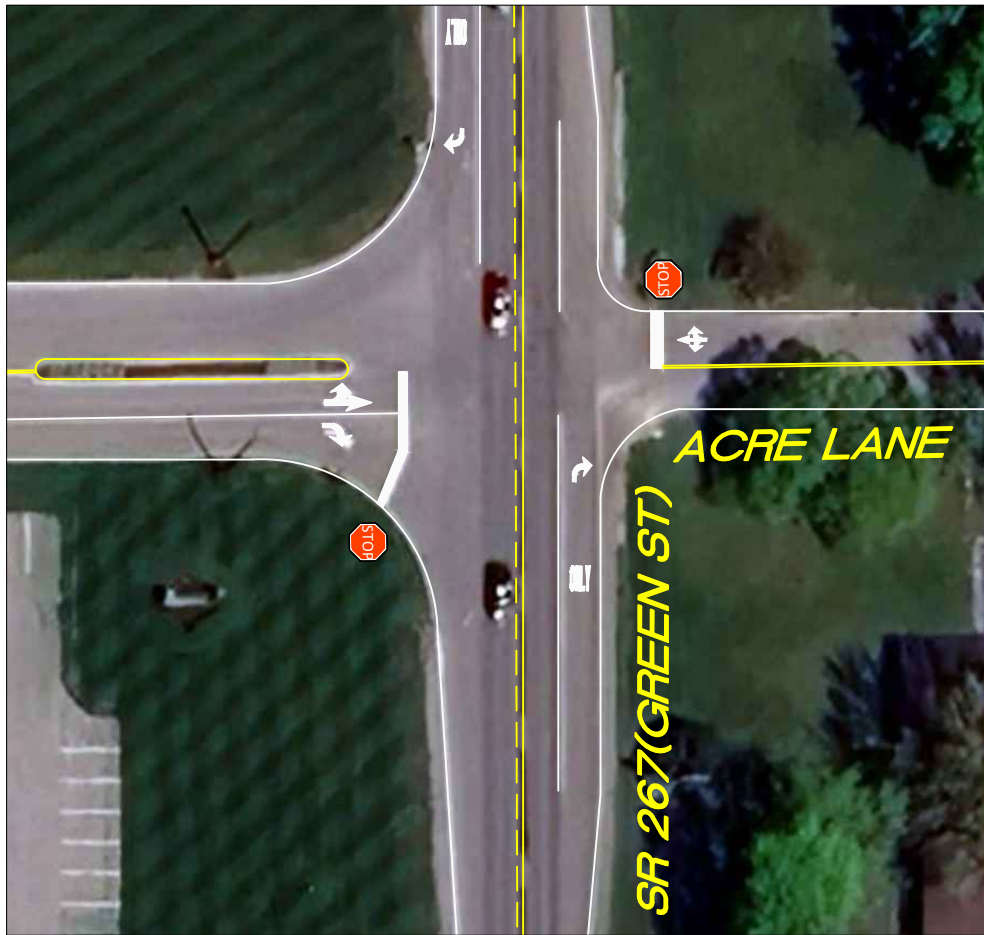


CR 800 N & SR 267 (GREEN ST)

FIGURE 2A

**EXISTING INTERSECTION
 GEOMETRICS**

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SR 267 (GREEN ST) & ACRE LANE

FIGURE 2B
EXISTING INTERSECTION
GEOMETRICS

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DESCRIPTION OF ABUTTING STREET SYSTEM

The proposed residential development will be primarily served by the public roadway system that includes CR 800 N, CR 650 E, CR 700 N, and SR 267 (Green Street).

TABLE 1 – DESCRIPTION OF THE ABUTTING STREET SYSTEM

STREET NAME	NUMBER OF LANES	SPEED LIMIT (MPH)	FUNCTIONAL CLASSIFICATION
CR 800 N	2	35	Collector
CR 650 E	2	45	Secondary Arterial
CR 700 N	2	30	Secondary Arterial
SR 267 (Green Street)	2	45	Primary Arterial

EXISTING TRAFFIC VOLUMES & PEAK HOURS

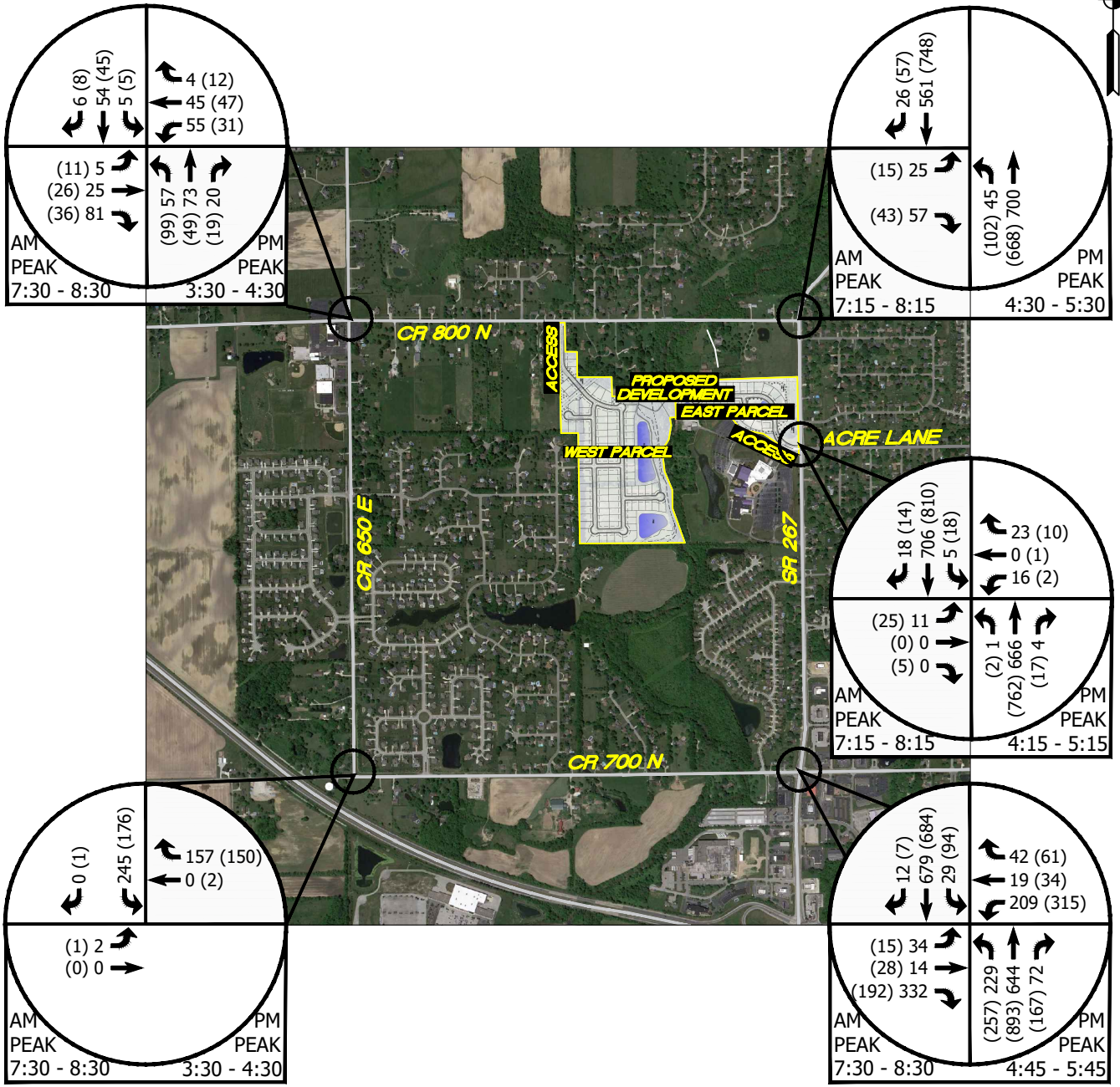
The turning movement traffic volume counts were collected by A&F Engineering at the study intersections between the hours of 6:30 AM to 9:00 AM and 3:30 PM to 7:00 PM during a typical weekday in December 2023 & May 2024 under good weather conditions. According to the turning movement counts, the AM and PM peak hours vary slightly at each study intersection. Hence, the actual peak hours are used at each study intersection to create a “worse-case” scenario. The peak hour volumes are shown on **Figure 3** and the intersection count output summary sheets, are included in the **Appendix**.

REDISTRIBUTION OF EXISTING TRAFFIC VOLUMES

The west parcel of the proposed development will have a secondary access via a connection to Windridge Lane. Therefore, it is possible that existing traffic from the Windridge subdivision could utilize this connection to access CR 800 N. However, it is not anticipated that these volumes will be significant during the peak hours. Therefore, these volumes have been considered negligible for the purposes of this analysis.

YEAR 2029 BACKGROUND TRAFFIC VOLUMES

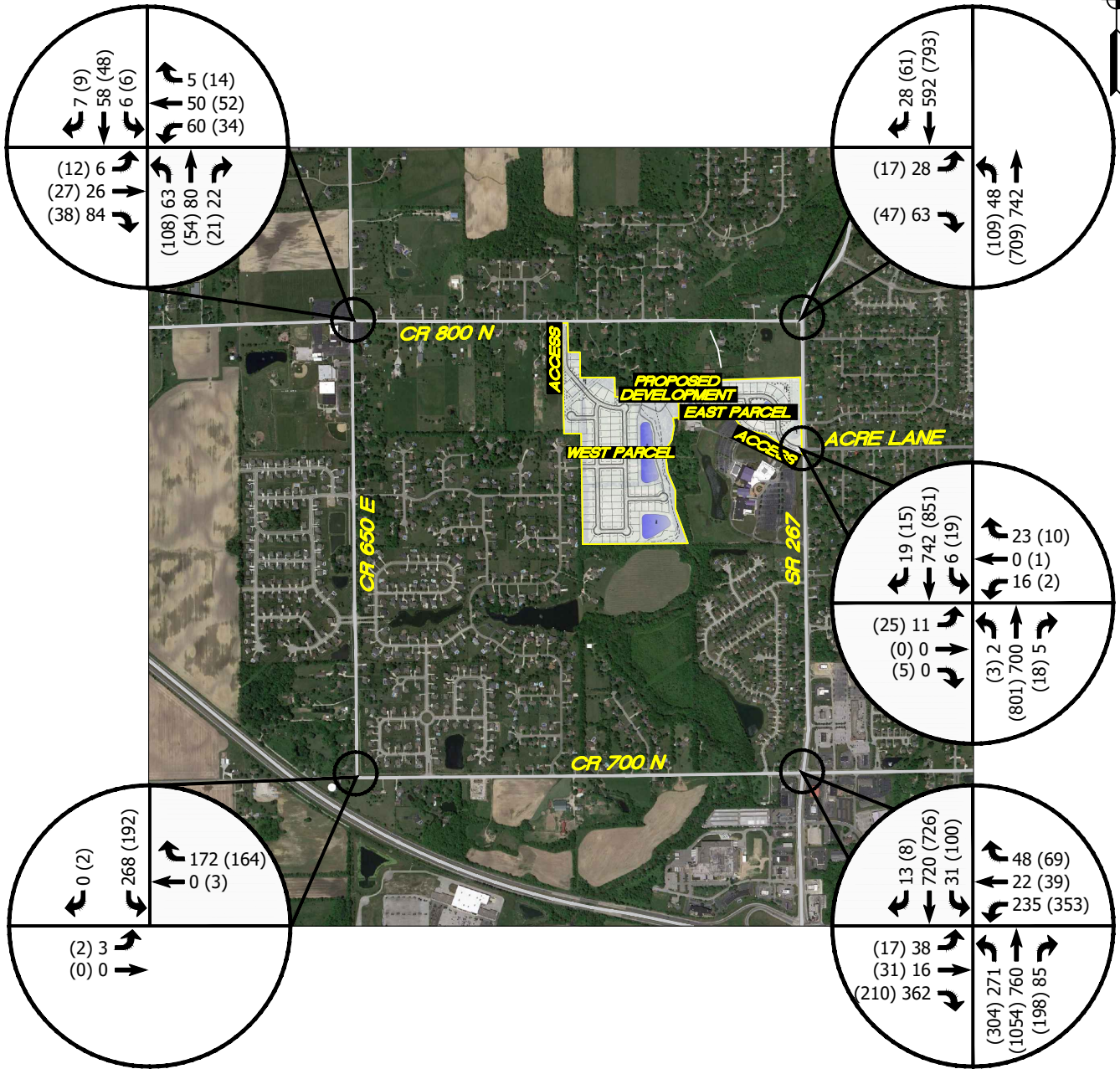
In order to account for annual growth in traffic that would occur due to future development outside of the study area, growth rate factors are applied to the existing traffic volumes. Growth rate factors provided by Shrewsbury were applied to the existing traffic volumes to obtain year 2029 background traffic volumes. The year 2029 background traffic volumes are shown in **Figure 4**.



LEGEND
 XX = A.M. PEAK HOUR
 (XX) = P.M. PEAK HOUR
 * = NEGLIGIBLE

FIGURE 3
EXISTING TRAFFIC VOLUMES

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LEGEND
 XX = A.M. PEAK HOUR
 (XX) = P.M. PEAK HOUR
 * = NEGLIGIBLE

FIGURE 4
YEAR 2029 BACKGROUND
TRAFFIC VOLUMES

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GENERATED TRIPS FOR PROPOSED DEVELOPMENT

The estimate of newly generated traffic is a function of the development size and of the character of the land use. The *ITE Trip Generation Manual*¹ was used to calculate the number of trips that will be generated by the proposed Hawk's Creek development. This report is a compilation of trip data for various land uses as collected by transportation professionals throughout the United States in order to establish the average number of trips generated by those land uses. **Table 2** summarizes the total number of trips that will be generated by the proposed development.

TABLE 2 – GENERATED TRIPS FOR PROPOSED DEVELOPMENT

DEVELOPMENT INFORMATION			GENERATED TRIPS			
LAND USE	ITE CODE	SIZE	AM PEAK HOUR		PM PEAK HOUR	
			ENTER	EXIT	ENTER	EXIT
Single-Family Housing (East Parcel)	210	25 DU	5	16	17	10
Single-Family Housing (West Parcel)	210	89 DU	17	50	56	33

PASS-BY & INTERNAL TRIPS

Pass-by trips are trips that are already in the existing traffic stream along the adjacent public roadway system that enter a site, utilize the site, and then return to the existing traffic stream. Residential developments do not typically attract a significant number of pass-by trips. Therefore, pass-by trip reductions are not included in this study.

An internal trip results when a trip is made between two or more land uses without traversing the external public roadway system. The proposed development is a single land use only. Hence, internal trip reductions are not considered in this study.

ASSIGNMENT AND DISTRIBUTION OF GENERATED TRIPS

The study methodology used to determine the traffic volumes from the site that will be added to the street system is defined as follows:

1. The volume of traffic that will enter and exit the proposed development must be assigned to the access points and to the public street system. Using the traffic volume data collected for this analysis, traffic to and from the site development has been assigned to the proposed driveways and to the public street system that will be serving the site.

¹ *Trip Generation Manual*, Institute of Transportation Engineers, Eleventh Edition, 2021.

- To determine the volume of traffic that will be added to the public roadway system, the generated traffic must be distributed by direction to the public roadways at their intersection with the driveways. For the proposed development, the trip distribution was based on the location of the development, the existing traffic patterns, and the assignment of generated traffic.

Figure 5A and 5B illustrate the assignment and distribution of generated traffic volumes for the proposed development.

GENERATED TRIPS ADDED TO THE STREET SYSTEM

The generated traffic volumes from the proposed development have been assigned to each of the study intersections. These volumes were determined based on the previously discussed trip generation data, assignment and distribution of generated traffic. The total peak hour generated traffic volumes from the proposed development are shown in **Figure 6**, and **Figure 7** shows the sum of the year 2029 background traffic volumes and generated traffic volumes from the proposed development.

TURN LANE WARRANT ANALYSIS

A turn lane analysis was conducted to determine if exclusive turn lanes would be warranted at the proposed access drive along CR 800 N when the proposed residential development is constructed. This analysis was done in accordance with the INDOT *Driveway Permit Manual*². The results are summarized in the following table.

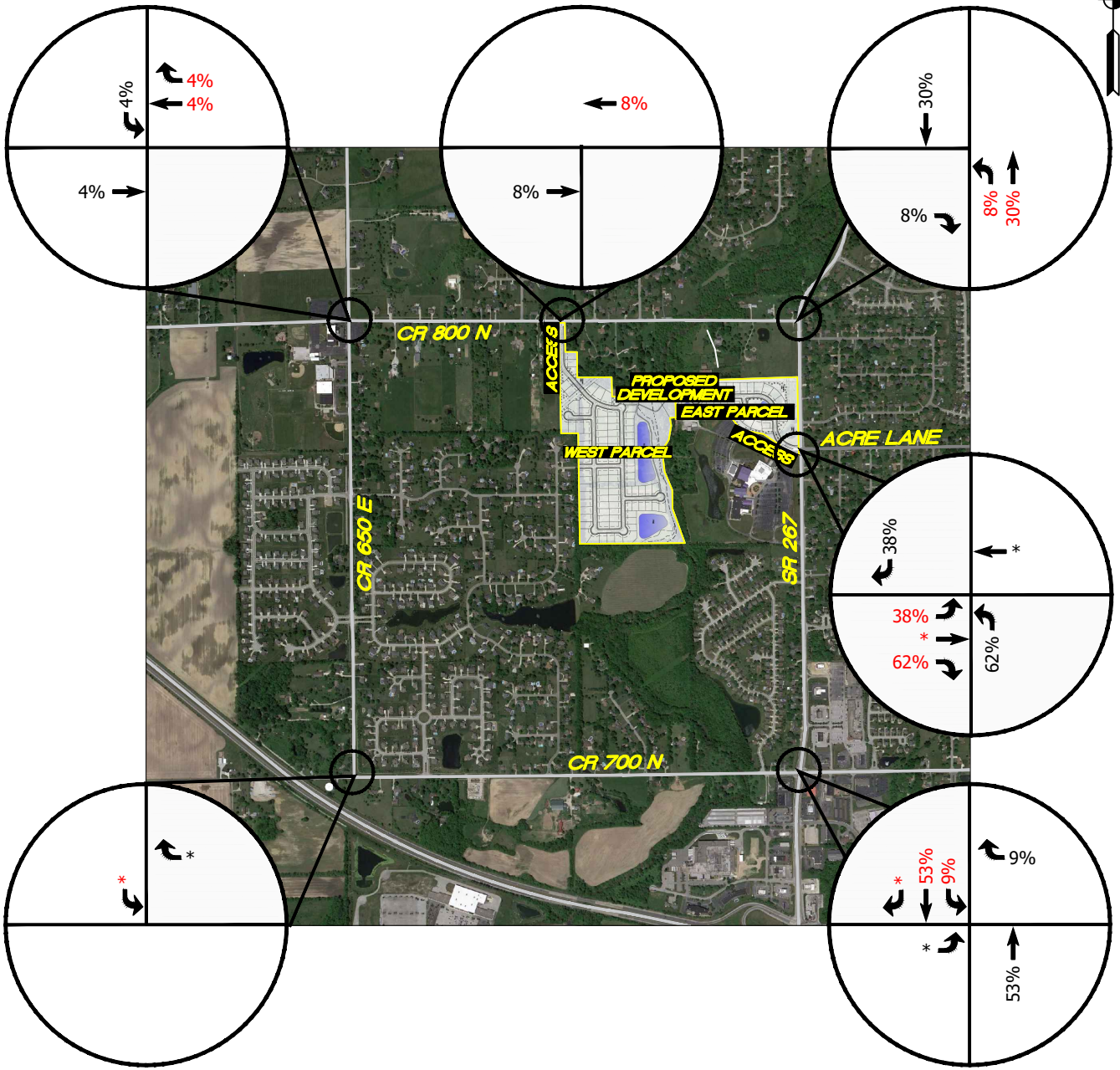
TABLE 3 –TURN LANE WARRANT ANALYSIS SUMMARY

LOCATION	SCENARIO	RIGHT-TURN LANE	LEFT-TURN LANE
CR 800 N & Proposed Development Access Drive	Year 2029 Traffic Volumes + Proposed Development Traffic Volumes	X	X

✓ =Turn Lane warranted; X =Turn Lane not warranted

The graphs that show the turn lane warrant criteria are shown in the **Appendix**.

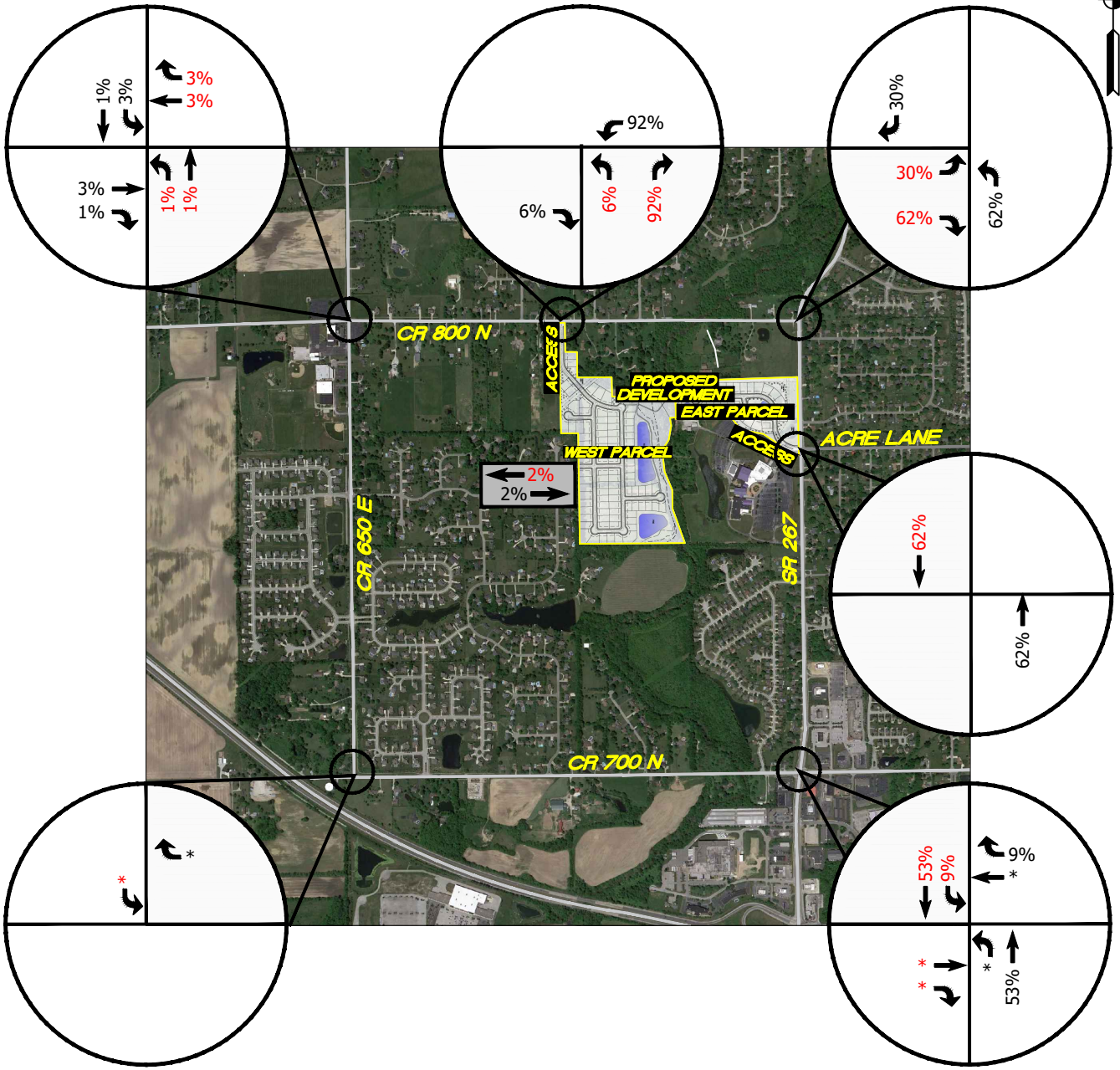
² INDOT *Driveway Permit Manual*, Indiana Department of Transportation, 2018



LEGEND
 XX = INBOUND TRAFFIC
 XX = OUTBOUND TRAFFIC
 * = NEGLIGIBLE

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BROWNSBURG, IN

FIGURE 5A
ASSIGNMENT & DISTRIBUTION OF
GENERATED TRAFFIC VOLUMES
FROM PROPOSED DEVELOPMENT
(EAST PARCEL)



LEGEND
 XX = INBOUND TRAFFIC
 XX = OUTBOUND TRAFFIC
 * = NEGLIGIBLE

FIGURE 5B

ASSIGNMENT & DISTRIBUTION OF GENERATED TRAFFIC VOLUMES FROM PROPOSED DEVELOPMENT (WEST PARCEL)

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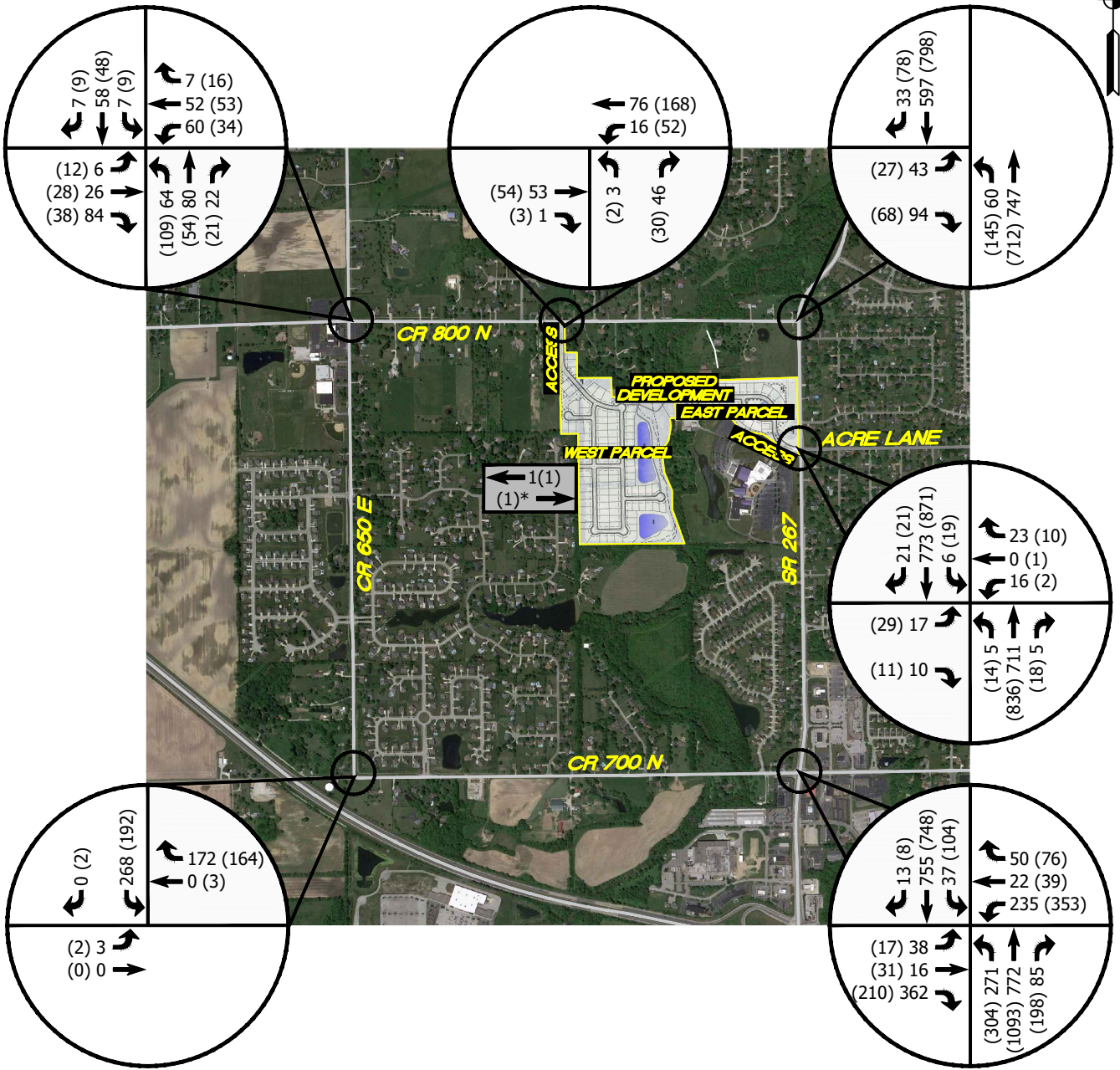


FIGURE 7

SUM OF YEAR 2029 BACKGROUND TRAFFIC VOLUMES & GENERATED TRAFFIC VOLUMES FROM PROPOSED DEVELOPMENT

**TRAFFIC IMPACT STUDY
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 BROWNSBURG, IN**

CAPACITY ANALYSIS

The "efficiency" of an intersection is based on its ability to accommodate the traffic volumes that approach the intersection. It is defined by the Level-of-Service (LOS) of the intersection. The LOS is determined by a series of calculations commonly called a "capacity analysis". Input data into a capacity analysis include traffic volumes, intersection geometry, and number and use of lanes. To determine the LOS at each of the study intersections, a capacity analysis has been made using the recognized computer program *Synchro/SimTraffic*³. This program allows intersections to be analyzed and optimized using the capacity calculation methods outlined within the *Highway Capacity Manual (HCM 7th Edition)*⁴. The following list shows the delays related to the levels of service for unsignalized and signalized intersections:

<u>Level of Service</u>	<u>Control Delay (seconds/vehicle)</u>	
	<u>UNSIGNALIZED</u>	<u>SIGNALIZED</u>
A	Less than or equal to 10	Less than or equal to 10
B	Between 10.1 and 15	Between 10.1 and 20
C	Between 15.1 and 25	Between 20.1 and 35
D	Between 25.1 and 35	Between 35.1 and 55
E	Between 35.1 and 50	Between 55.1 and 80
F	greater than 50	greater than 80

CAPACITY ANALYSIS SCENARIOS

To evaluate the proposed development's effect on the public street system, a series of traffic volume scenarios were analyzed to determine the adequacy of the existing roadway network. From this analysis, necessary recommendations can be made to improve the public street system so it will accommodate future traffic volumes. An analysis has been made for the peak hours at each of the study intersections for the following traffic volume scenarios:

Scenario 1: Existing Traffic Volumes – Based on existing peak hour traffic volumes.

Scenario 2: Year 2029 Background Traffic Volumes – Based on applying growth rate factors provided by Shrewsberry to the existing traffic volumes.

Scenario 3: Year 2029 + Proposed Development Traffic Volumes – Based on the sum of year 2029 background traffic volumes and generated traffic volumes from the proposed development.

³ *Synchro/SimTraffic 12*, Cubic Transportation Systems, 2023.

⁴ *Highway Capacity Manual (HCM), 7th Edition* Transportation Research Board, National Research Council, Washington, DC, 2022.

The following tables summarize the peak hour level of service results at each of the study intersections. The *Synchro (HCM 7th Edition)* intersection reports illustrating the capacity analysis results are included in the **Appendix**.

TABLE 4 – LEVEL OF SERVICE SUMMARY: CR 800 N & CR 650 E

APPROACH	AM PEAK			PM PEAK		
	Scenarios			Scenarios		
	1	2	3	1	2	3
Northbound Approach	B	B	B	A	A	A
Southbound Approach	A	A	A	A	A	A
Eastbound Approach	A	A	B	A	A	A
Westbound Approach	A	B	B	A	A	A
Intersection	A	B	B	A	A	A

TABLE 5 – LEVEL OF SERVICE SUMMARY: CR 800 N & SR 267 (GREEN STREET)

APPROACH	AM PEAK					PM PEAK				
	Scenarios					Scenarios				
	1	2A	2B	3A	3B	1	2A	2B	3A	3B
Northbound Left-Turn	A	A	A	A	A	B	B	B	B	B
Eastbound Approach	C	D	C	E	D	D	E	D	F	E

Analysis considers the following intersection geometrics:

Scenario A considers existing intersection geometrics.

Scenario B considers construction of a eastbound left-turn lane and right-turn lane.

TABLE 6 – LEVEL OF SERVICE SUMMARY: CR 700 N & SR 267 (GREEN STREET)

APPROACH	AM PEAK			PM PEAK		
	Scenarios			Scenarios		
	1	2	3	1	2	3
Northbound Approach	B	B	B	C	C	C
Southbound Approach	C	C	C	C	C	C
Eastbound Approach	C	C	C	C	C	C
Westbound Approach	C	C	C	C	D	D
Intersection	C	C	C	C	C	C

TABLE 7 – LEVEL OF SERVICE SUMMARY: CR 700 N & CR 650 E

APPROACH	AM PEAK			PM PEAK		
	Scenarios			Scenarios		
	1	2	3	1	2	3
Southbound Approach	B	B	B	B	B	B
Eastbound Left-Turn	A	A	A	A	A	A

TABLE 8 – LEVEL OF SERVICE SUMMARY: SR 267 (GREEN STREET) & ACRE LANE/CONNECTION
 POINTE CHRISTIAN CHURCH/PROPOSED DEVELOPMENT ACCESS DRIVE

APPROACH	AM PEAK			PM PEAK		
	Scenarios			Scenarios		
	1	2	3	1	2	3
Northbound Left-Turn	A	A	A	A	A	A
Southbound Left-Turn	A	A	A	A	A	A
Eastbound Approach	E	F	E	F	F	F
Westbound Approach	D	D	E	C	D	D

TABLE 9 – LEVEL OF SERVICE SUMMARY: CR 800 N & PROPOSED DEVELOPMENT ACCESS DRIVE

APPROACH	AM PEAK	PM PEAK
	Scenario 3	Scenario 3
Northbound Approach	A	A
Westbound Left-Turn	A	A

Analysis considers construction of the access drive with one inbound and one outbound lane that will stop for CR 800 N.

CONCLUSIONS & RECOMMENDATIONS

The conclusions that follow are based on existing traffic volume data, trip generation, assignment and distribution of generated traffic, capacity analyses/level of service results and a field review conducted at the site. Based on the analysis and the resulting conclusions of this study, recommendations are formulated to ensure that the roadway system will accommodate the increased traffic volumes from the proposed residential development.

CR 800 N & CR 650 E

Capacity analyses for all traffic volume scenarios have shown that this intersection currently operates and will continue to operate at acceptable levels of service during the AM and PM peak hours with the existing intersection conditions.

CR 800 N & SR 267 (GREEN STREET)

Capacity analyses have shown that all approaches to this intersection currently operate at acceptable levels of service during the AM and PM peak hours with the existing intersection conditions. It is expected that the eastbound approach will experience increased delays during the peak hours as future traffic is added to the intersection. However, additional analyses have shown that these delays can be substantially reduced if separate left-turn and right-turn lanes are constructed along the eastbound approach.

CR 700 N & SR 267 (GREEN STREET)

Capacity analyses for all traffic volume scenarios have shown that this intersection currently operates and will continue to operate at acceptable levels of service during the AM and PM peak hours with the existing intersection conditions.

CR 700 N & CR 650 E

Capacity analyses for all traffic volume scenarios have shown that all approaches to this intersection currently operate and will continue to operate at acceptable levels of service during the AM and PM peak hours with the existing intersection conditions.

SR 267 (GREEN STREET) & ACRE LANE/CONNECTION POINTE CHRISTIAN CHURCH/PROPOSED DEVELOPMENT ACCESS DRIVE

Capacity analyses have shown that the eastbound approach currently operates below acceptable levels of service during the peak hours. The eastbound and westbound approach delays will increase when future traffic volumes are added to the intersection. However, it should be noted that minor street approach peak hour delays are not uncommon when the major street has high peak hour volumes.

CR 800 N & PROPOSED DEVELOPMENT ACCESS DRIVE

Capacity analyses have shown that all approaches to this intersection will operate at acceptable levels of service during the AM and PM peak hours with the following recommended intersection conditions:

- Construction of the access drive with one inbound and one outbound lane.
- The intersection should be stop-controlled with the proposed access drive stopping for CR 800 N.

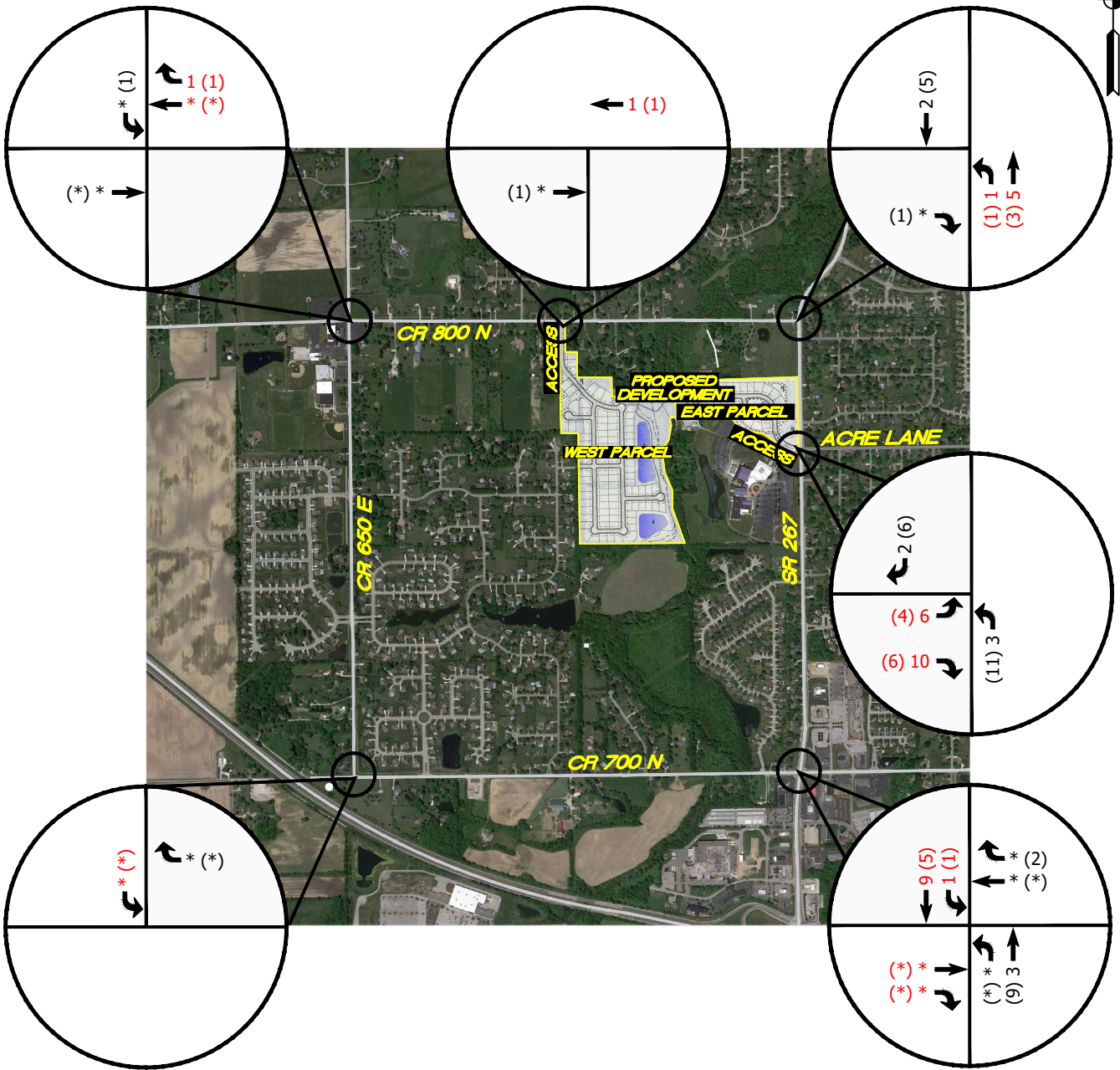
TRAFFIC IMPACT STUDY

APPENDIX



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ADDITIONAL FIGURES



LEGEND
 XX = A.M. PEAK HOUR
 (XX) = P.M. PEAK HOUR
 * = NEGLIGIBLE

FIGURE A
GENERATED TRAFFIC VOLUMES
FROM THE PROPOSED
DEVELOPMENT
(EAST PARCEL)

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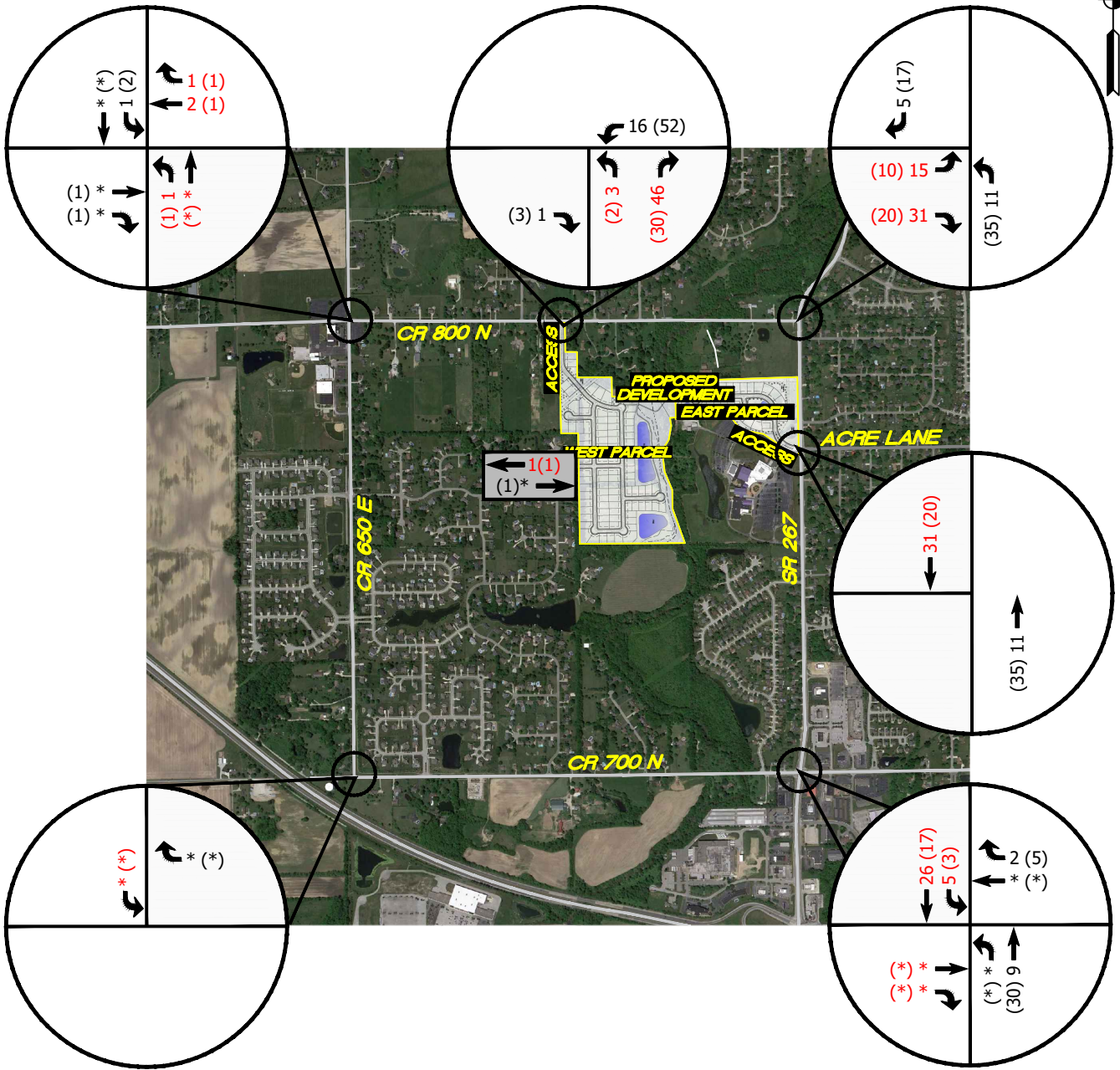


FIGURE B
GENERATED TRAFFIC VOLUMES
FROM THE PROPOSED
DEVELOPMENT
(WEST PARCEL)

TRAFFIC IMPACT STUDY
DAVID WEEKLEY HOMES
BROWNSBURG, IN

CR 800 N & CR 650 E

***TRAFFIC VOLUME COUNTS
CAPACITY ANALYSIS***

CR 800 N & CR 650 E - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142922, Location: 39.880045, -86.411371



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Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2023-12-12 7:30AM	1	13	4	0	18	2	11	1	0	14	2	5	11	0	18	2	7	2	0	11	61
7:45AM	7	9	0	0	16	1	6	0	0	7	0	9	15	0	24	14	4	0	0	18	65
8:00AM	18	20	7	0	45	2	17	2	0	21	1	5	27	0	33	19	13	1	0	33	132
8:15AM	31	31	9	0	71	0	20	3	0	23	2	6	28	0	36	20	21	1	0	42	172
Total	57	73	20	0	150	5	54	6	0	65	5	25	81	0	111	55	45	4	0	104	430
% Approach	38.0%	48.7%	13.3%	0%	-	7.7%	83.1%	9.2%	0%	-	4.5%	22.5%	73.0%	0%	-	52.9%	43.3%	3.8%	0%	-	-
% Total	13.3%	17.0%	4.7%	0%	34.9%	1.2%	12.6%	1.4%	0%	15.1%	1.2%	5.8%	18.8%	0%	25.8%	12.8%	10.5%	0.9%	0%	24.2%	-
PHF	0.460	0.589	0.556	-	0.528	0.625	0.675	0.500	-	0.707	0.625	0.694	0.723	-	0.771	0.688	0.536	0.500	-	0.619	0.625
Lights and Motorcycles	57	73	19	0	149	4	54	6	0	64	4	25	81	0	110	53	44	4	0	101	424
% Lights and Motorcycles	100%	100%	95.0%	0%	99.3%	80.0%	100%	100%	0%	98.5%	80.0%	100%	100%	0%	99.1%	96.4%	97.8%	100%	0%	97.1%	98.6%
Heavy	0	0	1	0	1	1	0	0	0	1	1	0	0	0	1	2	1	0	0	3	6
% Heavy	0%	0%	5.0%	0%	0.7%	20.0%	0%	0%	0%	1.5%	20.0%	0%	0%	0%	0.9%	3.6%	2.2%	0%	0%	2.9%	1.4%

*L: Left, R: Right, T: Thru, U: U-Turn

CR 800 N & CR 650 E - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

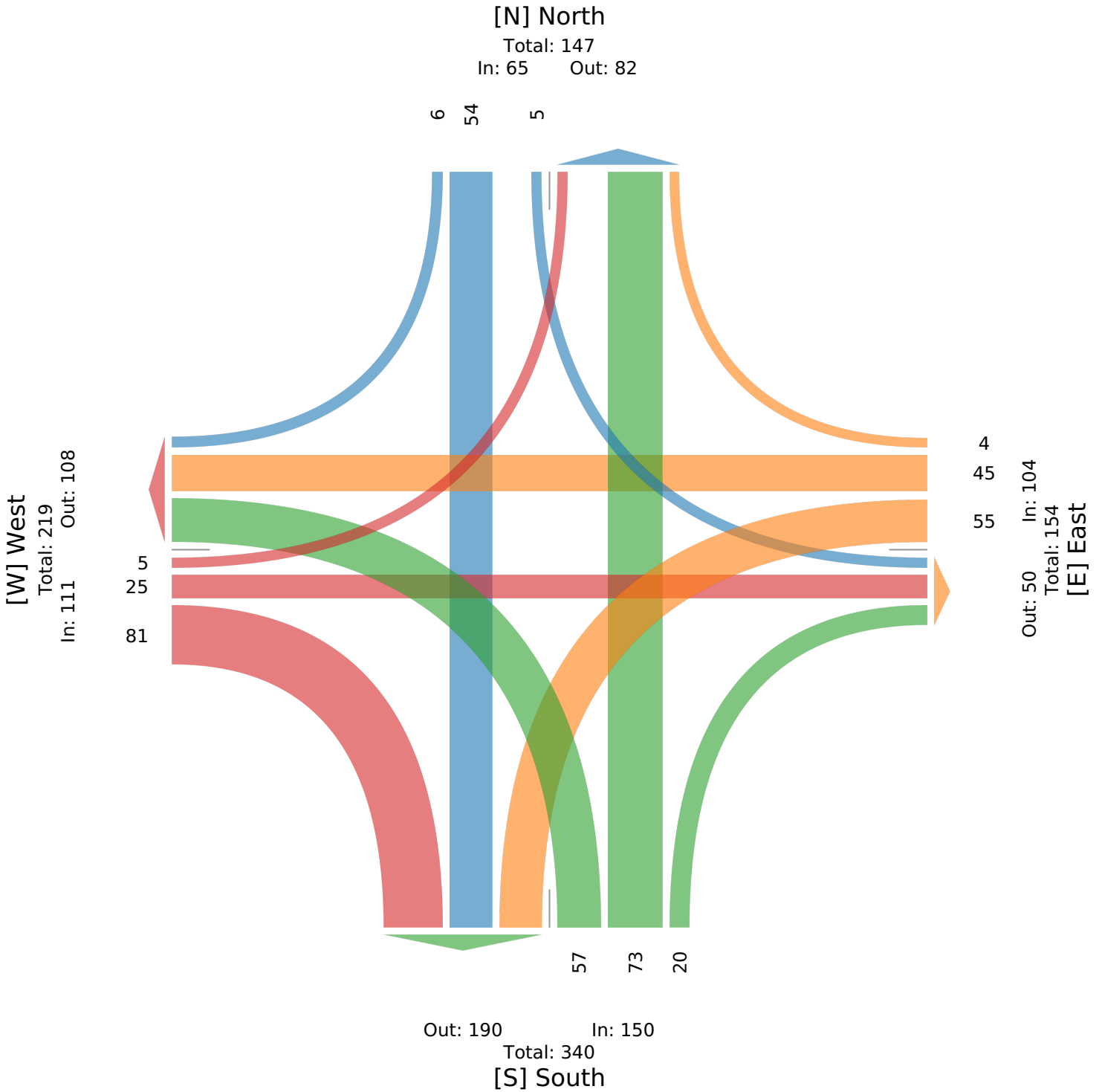
All Movements

ID: 1142922, Location: 39.880045, -86.411371



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



CR 800 N & CR 650 E - TMC

Tue Dec 12, 2023

PM Peak (3:30 PM - 4:30 PM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142922, Location: 39.880045, -86.411371



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2023-12-12 3:30PM	21	5	6	0	32	2	13	5	0	20	1	3	15	0	19	9	18	3	0	30	101
3:45PM	51	20	9	0	80	0	10	1	0	11	4	8	5	0	17	9	14	1	0	24	132
4:00PM	14	14	2	0	30	0	9	1	0	10	3	4	8	0	15	5	7	3	0	15	70
4:15PM	13	10	2	0	25	3	13	1	0	17	3	11	8	0	22	8	8	5	0	21	85
Total	99	49	19	0	167	5	45	8	0	58	11	26	36	0	73	31	47	12	0	90	388
% Approach	59.3%	29.3%	11.4%	0%	-	8.6%	77.6%	13.8%	0%	-	15.1%	35.6%	49.3%	0%	-	34.4%	52.2%	13.3%	0%	-	-
% Total	25.5%	12.6%	4.9%	0%	43.0%	1.3%	11.6%	2.1%	0%	14.9%	2.8%	6.7%	9.3%	0%	18.8%	8.0%	12.1%	3.1%	0%	23.2%	-
PHF	0.485	0.613	0.528	-	0.522	0.417	0.865	0.400	-	0.725	0.688	0.591	0.600	-	0.830	0.861	0.653	0.600	-	0.750	0.735
Lights and Motorcycles	99	47	19	0	165	5	43	8	0	56	9	24	36	0	69	28	46	9	0	83	373
% Lights and Motorcycles	100%	95.9%	100%	0%	98.8%	100%	95.6%	100%	0%	96.6%	81.8%	92.3%	100%	0%	94.5%	90.3%	97.9%	75.0%	0%	92.2%	96.1%
Heavy	0	2	0	0	2	0	2	0	0	2	2	2	0	0	4	3	1	3	0	7	15
% Heavy	0%	4.1%	0%	0%	1.2%	0%	4.4%	0%	0%	3.4%	18.2%	7.7%	0%	0%	5.5%	9.7%	2.1%	25.0%	0%	7.8%	3.9%

*L: Left, R: Right, T: Thru, U: U-Turn

CR 800 N & CR 650 E - TMC

Tue Dec 12, 2023

PM Peak (3:30 PM - 4:30 PM)

All Classes (Lights and Motorcycles, Heavy)

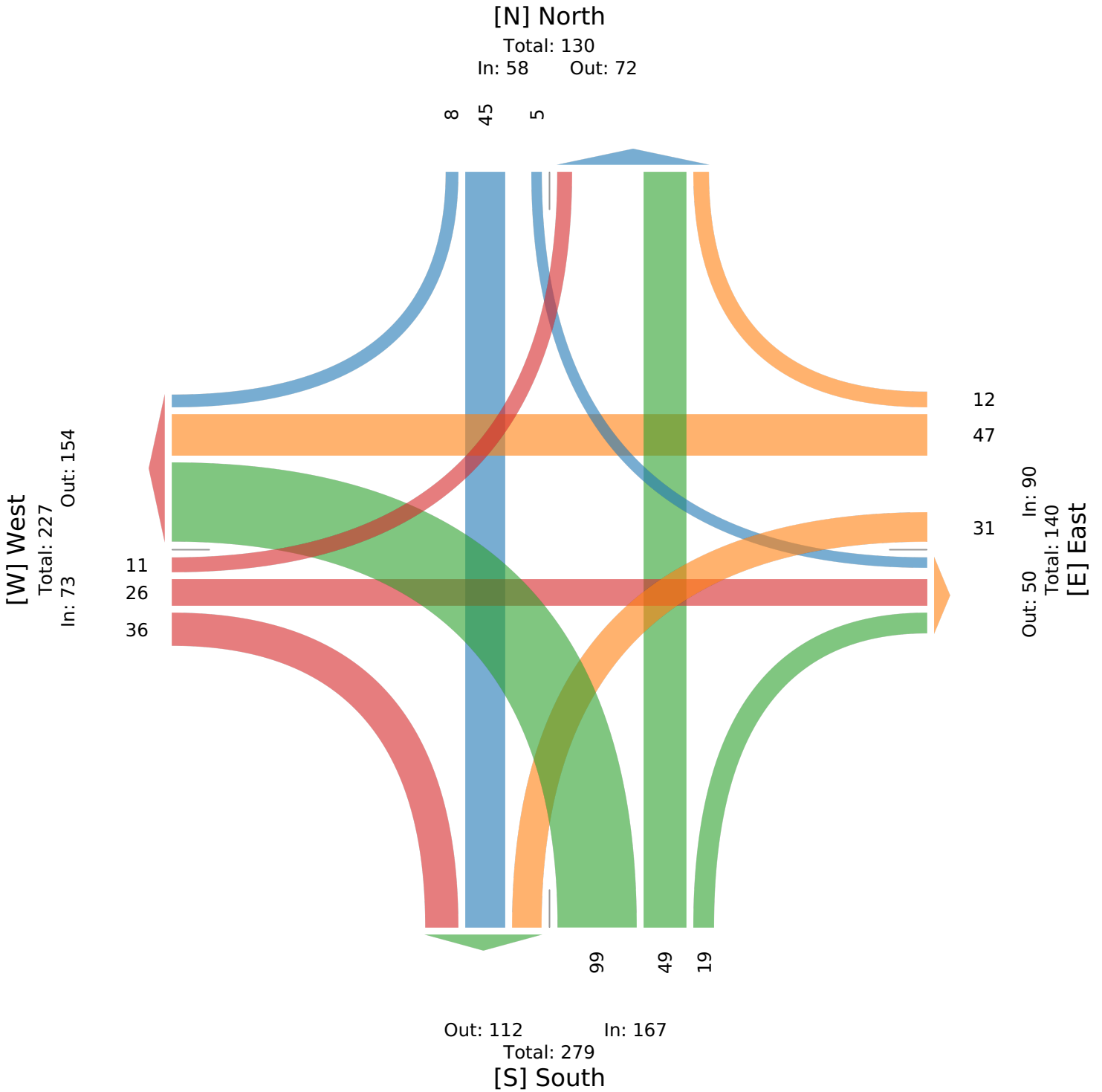
All Movements

ID: 1142922, Location: 39.880045, -86.411371



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



Intersection	
Intersection Delay, s/veh	9.9
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	25	81	55	45	4	57	73	20	5	54	6
Future Vol, veh/h	5	25	81	55	45	4	57	73	20	5	54	6
Peak Hour Factor	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62
Heavy Vehicles, %	20	0	0	4	2	0	0	0	5	20	0	0
Mvmt Flow	8	40	131	89	73	6	92	118	32	8	87	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	9.6	9.9	10.4	9.5
HCM LOS	A	A	B	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	38%	5%	53%	8%
Vol Thru, %	49%	23%	43%	83%
Vol Right, %	13%	73%	4%	9%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	150	111	104	65
LT Vol	57	5	55	5
Through Vol	73	25	45	54
RT Vol	20	81	4	6
Lane Flow Rate	242	179	168	105
Geometry Grp	1	1	1	1
Degree of Util (X)	0.329	0.244	0.24	0.157
Departure Headway (Hd)	4.9	4.908	5.152	5.381
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	726	723	689	659
Service Time	2.977	2.99	3.236	3.474
HCM Lane V/C Ratio	0.333	0.248	0.244	0.159
HCM Control Delay, s/veh	10.4	9.6	9.9	9.5
HCM Lane LOS	B	A	A	A
HCM 95th-tile Q	1.4	1	0.9	0.6

Intersection	
Intersection Delay, s/veh	9
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	11	26	36	31	47	12	99	49	19	5	45	8
Future Vol, veh/h	11	26	36	31	47	12	99	49	19	5	45	8
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles, %	18	8	0	10	2	25	0	4	0	0	4	0
Mvmt Flow	15	35	49	42	64	16	134	66	26	7	61	11
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	8.6	8.9	9.5	8.2
HCM LOS	A	A	A	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	59%	15%	34%	9%
Vol Thru, %	29%	36%	52%	78%
Vol Right, %	11%	49%	13%	14%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	167	73	90	58
LT Vol	99	11	31	5
Through Vol	49	26	47	45
RT Vol	19	36	12	8
Lane Flow Rate	226	99	122	78
Geometry Grp	1	1	1	1
Degree of Util (X)	0.287	0.132	0.165	0.101
Departure Headway (Hd)	4.585	4.811	4.897	4.643
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	783	744	731	770
Service Time	2.617	2.851	2.937	2.683
HCM Lane V/C Ratio	0.289	0.133	0.167	0.101
HCM Control Delay, s/veh	9.5	8.6	8.9	8.2
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	1.2	0.5	0.6	0.3

Intersection	
Intersection Delay, s/veh	10.5
Intersection LOS	B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	6	26	84	60	50	5	63	80	22	6	58	7
Future Vol, veh/h	6	26	84	60	50	5	63	80	22	6	58	7
Peak Hour Factor	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62
Heavy Vehicles, %	20	0	0	4	2	0	0	0	5	20	0	0
Mvmt Flow	10	42	135	97	81	8	102	129	35	10	94	11
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	10	10.4	11.2	9.9
HCM LOS	A	B	B	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	38%	5%	52%	8%
Vol Thru, %	48%	22%	43%	82%
Vol Right, %	13%	72%	4%	10%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	165	116	115	71
LT Vol	63	6	60	6
Through Vol	80	26	50	58
RT Vol	22	84	5	7
Lane Flow Rate	266	187	185	115
Geometry Grp	1	1	1	1
Degree of Util (X)	0.378	0.267	0.276	0.179
Departure Headway (Hd)	5.114	5.143	5.366	5.616
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	708	698	670	639
Service Time	3.114	3.175	3.398	3.646
HCM Lane V/C Ratio	0.376	0.268	0.276	0.18
HCM Control Delay, s/veh	11.2	10	10.4	9.9
HCM Lane LOS	B	A	B	A
HCM 95th-tile Q	1.8	1.1	1.1	0.6

Intersection	
Intersection Delay, s/veh	9.3
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	12	27	38	34	52	14	108	54	21	6	48	9
Future Vol, veh/h	12	27	38	34	52	14	108	54	21	6	48	9
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles, %	18	8	0	10	2	25	0	4	0	0	4	0
Mvmt Flow	16	36	51	46	70	19	146	73	28	8	65	12
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	8.8	9.2	9.9	8.4
HCM LOS	A	A	A	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	59%	16%	34%	10%
Vol Thru, %	30%	35%	52%	76%
Vol Right, %	11%	49%	14%	14%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	183	77	100	63
LT Vol	108	12	34	6
Through Vol	54	27	52	48
RT Vol	21	38	14	9
Lane Flow Rate	247	104	135	85
Geometry Grp	1	1	1	1
Degree of Util (X)	0.319	0.142	0.187	0.112
Departure Headway (Hd)	4.647	4.905	4.974	4.727
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	772	727	719	754
Service Time	2.688	2.959	3.025	2.779
HCM Lane V/C Ratio	0.32	0.143	0.188	0.113
HCM Control Delay, s/veh	9.9	8.8	9.2	8.4
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	1.4	0.5	0.7	0.4

Intersection	
Intersection Delay, s/veh	10.6
Intersection LOS	B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	6	26	84	60	52	7	64	80	22	7	58	7
Future Vol, veh/h	6	26	84	60	52	7	64	80	22	7	58	7
Peak Hour Factor	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62
Heavy Vehicles, %	20	0	0	4	2	0	0	0	5	20	0	0
Mvmt Flow	10	42	135	97	84	11	103	129	35	11	94	11
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	10.1	10.6	11.3	9.9
HCM LOS	B	B	B	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	39%	5%	50%	10%
Vol Thru, %	48%	22%	44%	81%
Vol Right, %	13%	72%	6%	10%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	166	116	119	72
LT Vol	64	6	60	7
Through Vol	80	26	52	58
RT Vol	22	84	7	7
Lane Flow Rate	268	187	192	116
Geometry Grp	1	1	1	1
Degree of Util (X)	0.382	0.269	0.286	0.182
Departure Headway (Hd)	5.141	5.167	5.368	5.644
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	704	696	670	635
Service Time	3.141	3.2	3.401	3.679
HCM Lane V/C Ratio	0.381	0.269	0.287	0.183
HCM Control Delay, s/veh	11.3	10.1	10.6	9.9
HCM Lane LOS	B	B	B	A
HCM 95th-tile Q	1.8	1.1	1.2	0.7

Intersection	
Intersection Delay, s/veh	9.3
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	12	28	39	34	53	16	109	54	21	9	48	9
Future Vol, veh/h	12	28	39	34	53	16	109	54	21	9	48	9
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles, %	18	8	0	10	2	25	0	4	0	0	4	0
Mvmt Flow	16	38	53	46	72	22	147	73	28	12	65	12
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay, s/veh	8.8	9.2	9.9	8.5
HCM LOS	A	A	A	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	59%	15%	33%	14%
Vol Thru, %	29%	35%	51%	73%
Vol Right, %	11%	49%	16%	14%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	184	79	103	66
LT Vol	109	12	34	9
Through Vol	54	28	53	48
RT Vol	21	39	16	9
Lane Flow Rate	249	107	139	89
Geometry Grp	1	1	1	1
Degree of Util (X)	0.323	0.146	0.193	0.118
Departure Headway (Hd)	4.671	4.927	4.984	4.761
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	768	724	717	749
Service Time	2.714	2.984	3.037	2.815
HCM Lane V/C Ratio	0.324	0.148	0.194	0.119
HCM Control Delay, s/veh	9.9	8.8	9.2	8.5
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	1.4	0.5	0.7	0.4

CR 800 N & SR 267

TRAFFIC VOLUME COUNTS CAPACITY ANALYSIS

SR 267 & CR 800 N - TMC

Tue Dec 12, 2023

AM Peak (7:15 AM - 8:15 AM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142924, Location: 39.880075, -86.392508



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound				North Southbound				West Eastbound				Int
	L	T	U	App	T	R	U	App	L	R	U	App	
2023-12-12 7:15AM	5	191	0	196	156	1	0	157	10	13	0	23	376
7:30AM	8	191	0	199	131	5	0	136	9	12	0	21	356
7:45AM	4	182	0	186	145	10	0	155	4	17	0	21	362
8:00AM	28	136	0	164	129	10	0	139	2	15	0	17	320
Total	45	700	0	745	561	26	0	587	25	57	0	82	1414
% Approach	6.0%	94.0%	0%	-	95.6%	4.4%	0%	-	30.5%	69.5%	0%	-	-
% Total	3.2%	49.5%	0%	52.7%	39.7%	1.8%	0%	41.5%	1.8%	4.0%	0%	5.8%	-
PHF	0.402	0.916	-	0.936	0.899	0.650	-	0.935	0.625	0.838	-	0.891	0.940
Lights and Motorcycles	45	680	0	725	540	24	0	564	25	55	0	80	1369
% Lights and Motorcycles	100%	97.1%	0%	97.3%	96.3%	92.3%	0%	96.1%	100%	96.5%	0%	97.6%	96.8%
Heavy	0	20	0	20	21	2	0	23	0	2	0	2	45
% Heavy	0%	2.9%	0%	2.7%	3.7%	7.7%	0%	3.9%	0%	3.5%	0%	2.4%	3.2%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & CR 800 N - TMC

Tue Dec 12, 2023

AM Peak (7:15 AM - 8:15 AM)

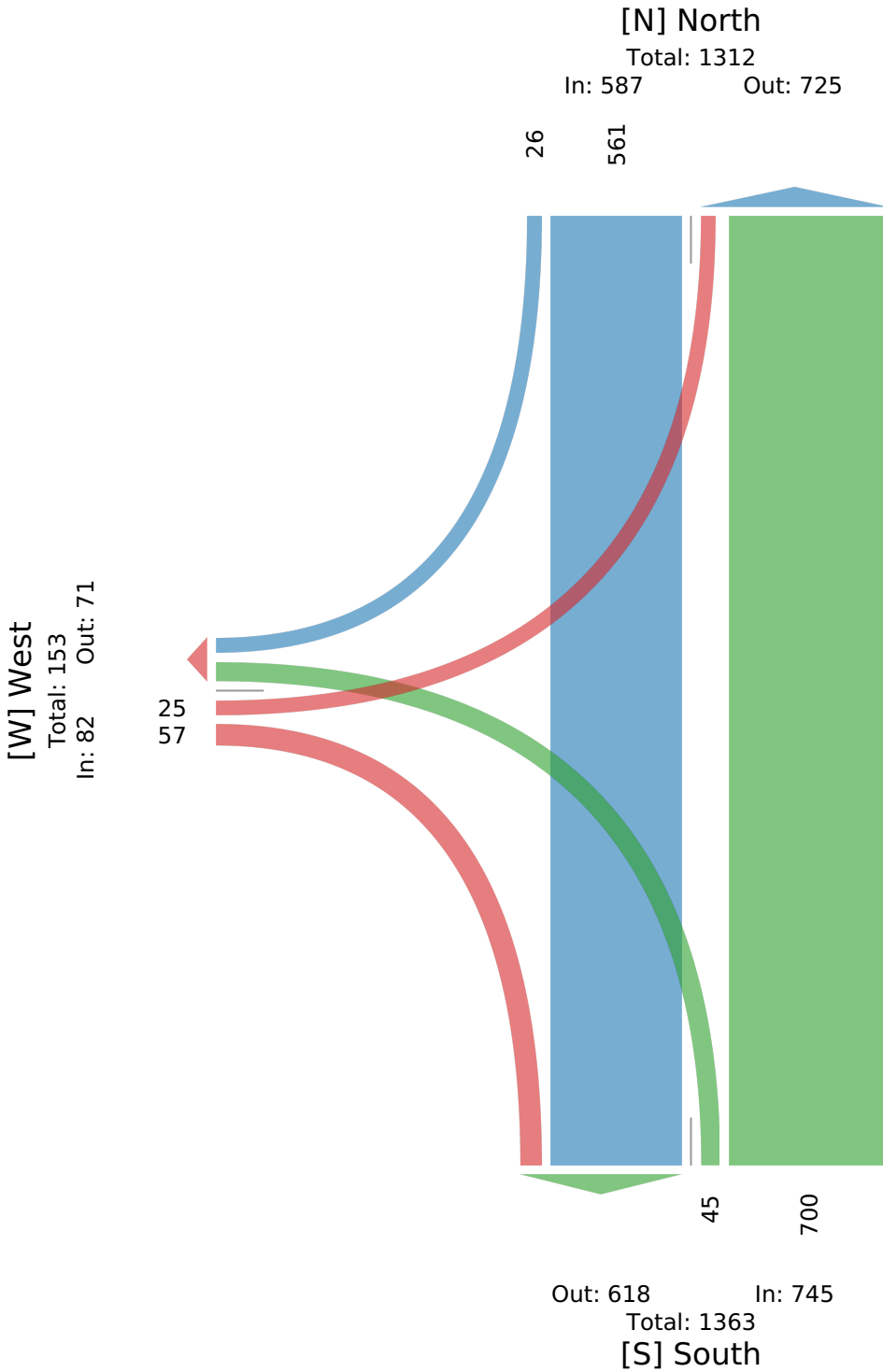
All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142924, Location: 39.880075, -86.392508



Provided by: A&F Engineering
8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



SR 267 & CR 800 N - TMC

Tue Dec 12, 2023

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142924, Location: 39.880075, -86.392508



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound				North Southbound				West Eastbound				Int
	L	T	U	App	T	R	U	App	L	R	U	App	
2023-12-12 4:30PM	30	169	0	199	172	18	0	190	3	5	0	8	397
4:45PM	20	156	0	176	202	13	0	215	1	10	0	11	402
5:00PM	26	173	0	199	174	15	0	189	7	19	0	26	414
5:15PM	26	170	0	196	200	11	0	211	4	9	0	13	420
Total	102	668	0	770	748	57	0	805	15	43	0	58	1633
% Approach	13.2%	86.8%	0%	-	92.9%	7.1%	0%	-	25.9%	74.1%	0%	-	-
% Total	6.2%	40.9%	0%	47.2%	45.8%	3.5%	0%	49.3%	0.9%	2.6%	0%	3.6%	-
PHF	0.850	0.965	-	0.967	0.926	0.792	-	0.936	0.536	0.566	-	0.558	0.972
Lights and Motorcycles	102	655	0	757	726	56	0	782	14	43	0	57	1596
% Lights and Motorcycles	100%	98.1%	0%	98.3%	97.1%	98.2%	0%	97.1%	93.3%	100%	0%	98.3%	97.7%
Heavy	0	13	0	13	22	1	0	23	1	0	0	1	37
% Heavy	0%	1.9%	0%	1.7%	2.9%	1.8%	0%	2.9%	6.7%	0%	0%	1.7%	2.3%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & CR 800 N - TMC

Tue Dec 12, 2023

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

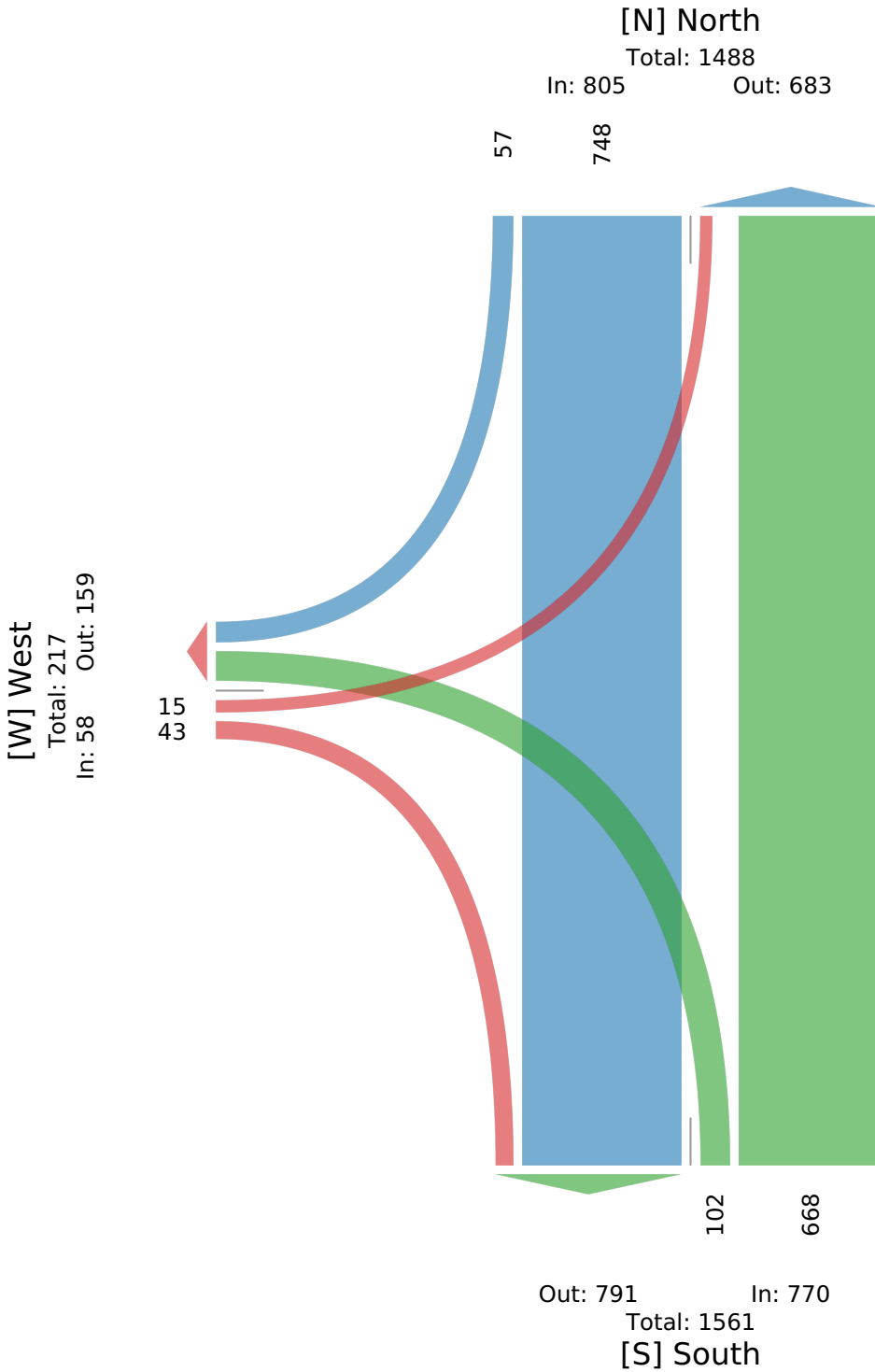
All Movements

ID: 1142924, Location: 39.880075, -86.392508



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	Y
Traffic Vol, veh/h	25	57	45	700	561	26
Future Vol, veh/h	25	57	45	700	561	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	4	0	3	4	8
Mvmt Flow	27	61	48	745	597	28

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1437	597	624	0	-	0
Stage 1	597	-	-	-	-	-
Stage 2	840	-	-	-	-	-
Critical Hdwy	6.4	6.24	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	2.2	-	-	-
Pot Cap-1 Maneuver	148	499	967	-	-	-
Stage 1	554	-	-	-	-	-
Stage 2	427	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	141	499	967	-	-	-
Mov Cap-2 Maneuver	141	-	-	-	-	-
Stage 1	526	-	-	-	-	-
Stage 2	427	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v	23.45	0.54	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	967	-	281	-	-
HCM Lane V/C Ratio	0.05	-	0.31	-	-
HCM Control Delay (s/veh)	8.9	-	23.4	-	-
HCM Lane LOS	A	-	C	-	-
HCM 95th %tile Q(veh)	0.2	-	1.3	-	-

Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	↑
Traffic Vol, veh/h	15	43	102	668	748	57
Future Vol, veh/h	15	43	102	668	748	57
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	7	0	0	2	3	2
Mvmt Flow	15	44	105	689	771	59

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1670	771	830	0	-	0
Stage 1	771	-	-	-	-	-
Stage 2	899	-	-	-	-	-
Critical Hdwy	6.47	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	103	403	811	-	-	-
Stage 1	448	-	-	-	-	-
Stage 2	389	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	89	403	811	-	-	-
Mov Cap-2 Maneuver	89	-	-	-	-	-
Stage 1	390	-	-	-	-	-
Stage 2	389	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v	28.63	1.34	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	811	-	211	-	-
HCM Lane V/C Ratio	0.13	-	0.283	-	-
HCM Control Delay (s/veh)	10.1	-	28.6	-	-
HCM Lane LOS	B	-	D	-	-
HCM 95th %tile Q(veh)	0.4	-	1.1	-	-

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	Y
Traffic Vol, veh/h	28	63	48	742	592	28
Future Vol, veh/h	28	63	48	742	592	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	4	0	3	4	8
Mvmt Flow	30	67	51	789	630	30

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1521	630	660	0	-	0
Stage 1	630	-	-	-	-	-
Stage 2	891	-	-	-	-	-
Critical Hdwy	6.4	6.24	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	2.2	-	-	-
Pot Cap-1 Maneuver	132	478	938	-	-	-
Stage 1	535	-	-	-	-	-
Stage 2	404	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	125	478	938	-	-	-
Mov Cap-2 Maneuver	125	-	-	-	-	-
Stage 1	506	-	-	-	-	-
Stage 2	404	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v	27.46	0.55	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	938	-	255	-	-
HCM Lane V/C Ratio	0.054	-	0.379	-	-
HCM Control Delay (s/veh)	9.1	-	27.5	-	-
HCM Lane LOS	A	-	D	-	-
HCM 95th %tile Q(veh)	0.2	-	1.7	-	-

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	Y
Traffic Vol, veh/h	17	47	109	709	793	61
Future Vol, veh/h	17	47	109	709	793	61
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	7	0	0	2	3	2
Mvmt Flow	18	48	112	731	818	63

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1773	818	880	0	-	0
Stage 1	818	-	-	-	-	-
Stage 2	956	-	-	-	-	-
Critical Hdwy	6.47	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	89	379	776	-	-	-
Stage 1	426	-	-	-	-	-
Stage 2	366	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	76	379	776	-	-	-
Mov Cap-2 Maneuver	76	-	-	-	-	-
Stage 1	364	-	-	-	-	-
Stage 2	366	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v35.19		1.39	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	776	-	184	-	-
HCM Lane V/C Ratio	0.145	-	0.359	-	-
HCM Control Delay (s/veh)	10.4	-	35.2	-	-
HCM Lane LOS	B	-	E	-	-
HCM 95th %tile Q(veh)	0.5	-	1.5	-	-

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘	↗	↘	↗	↗	↗
Traffic Vol, veh/h	28	63	48	742	592	28
Future Vol, veh/h	28	63	48	742	592	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	4	0	3	4	8
Mvmt Flow	30	67	51	789	630	30

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1521	630	660	0	-	0
Stage 1	630	-	-	-	-	-
Stage 2	891	-	-	-	-	-
Critical Hdwy	6.4	6.24	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	2.2	-	-	-
Pot Cap-1 Maneuver	132	478	938	-	-	-
Stage 1	535	-	-	-	-	-
Stage 2	404	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	125	478	938	-	-	-
Mov Cap-2 Maneuver	125	-	-	-	-	-
Stage 1	506	-	-	-	-	-
Stage 2	404	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v22.67		0.55	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	938	-	125	478	-	-
HCM Lane V/C Ratio	0.054	-	0.239	0.14	-	-
HCM Control Delay (s/veh)	9.1	-	42.7	13.8	-	-
HCM Lane LOS	A	-	E	B	-	-
HCM 95th %tile Q(veh)	0.2	-	0.9	0.5	-	-

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘	↗	↘	↗	↗	↗
Traffic Vol, veh/h	17	47	109	709	793	61
Future Vol, veh/h	17	47	109	709	793	61
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	7	0	0	2	3	2
Mvmt Flow	18	48	112	731	818	63

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1773	818	880	0	-	0
Stage 1	818	-	-	-	-	-
Stage 2	956	-	-	-	-	-
Critical Hdwy	6.47	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	89	379	776	-	-	-
Stage 1	426	-	-	-	-	-
Stage 2	366	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	76	379	776	-	-	-
Mov Cap-2 Maneuver	76	-	-	-	-	-
Stage 1	364	-	-	-	-	-
Stage 2	366	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v29.27		1.39	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	776	-	76	379	-	-
HCM Lane V/C Ratio	0.145	-	0.231	0.128	-	-
HCM Control Delay (s/veh)	10.4	-	66.3	15.9	-	-
HCM Lane LOS	B	-	F	C	-	-
HCM 95th %tile Q(veh)	0.5	-	0.8	0.4	-	-

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	Y
Traffic Vol, veh/h	43	94	60	747	597	33
Future Vol, veh/h	43	94	60	747	597	33
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	4	0	3	4	8
Mvmt Flow	46	100	64	795	635	35

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1557	635	670	0	-	0
Stage 1	635	-	-	-	-	-
Stage 2	922	-	-	-	-	-
Critical Hdwy	6.4	6.24	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	2.2	-	-	-
Pot Cap-1 Maneuver	125	475	930	-	-	-
Stage 1	532	-	-	-	-	-
Stage 2	391	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	117	475	930	-	-	-
Mov Cap-2 Maneuver	117	-	-	-	-	-
Stage 1	495	-	-	-	-	-
Stage 2	391	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v40.17		0.68	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	930	-	242	-	-
HCM Lane V/C Ratio	0.069	-	0.603	-	-
HCM Control Delay (s/veh)	9.2	-	40.2	-	-
HCM Lane LOS	A	-	E	-	-
HCM 95th %tile Q(veh)	0.2	-	3.5	-	-

Intersection						
Int Delay, s/veh	4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		Y	↑	↑	Y
Traffic Vol, veh/h	27	68	145	712	798	78
Future Vol, veh/h	27	68	145	712	798	78
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	7	0	0	2	3	2
Mvmt Flow	28	70	149	734	823	80

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1856	823	903	0	-	0
Stage 1	823	-	-	-	-	-
Stage 2	1033	-	-	-	-	-
Critical Hdwy	6.47	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	79	377	761	-	-	-
Stage 1	423	-	-	-	-	-
Stage 2	336	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	63	377	761	-	-	-
Mov Cap-2 Maneuver	63	-	-	-	-	-
Stage 1	340	-	-	-	-	-
Stage 2	336	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v60.45		1.84	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	761	-	156	-	-
HCM Lane V/C Ratio	0.196	-	0.627	-	-
HCM Control Delay (s/veh)	10.9	-	60.5	-	-
HCM Lane LOS	B	-	F	-	-
HCM 95th %tile Q(veh)	0.7	-	3.4	-	-

Intersection						
Int Delay, s/veh	2.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↙	↗	↙	↗	↗	↗
Traffic Vol, veh/h	43	94	60	747	597	33
Future Vol, veh/h	43	94	60	747	597	33
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	4	0	3	4	8
Mvmt Flow	46	100	64	795	635	35

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1557	635	670	0	-	0
Stage 1	635	-	-	-	-	-
Stage 2	922	-	-	-	-	-
Critical Hdwy	6.4	6.24	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	2.2	-	-	-
Pot Cap-1 Maneuver	125	475	930	-	-	-
Stage 1	532	-	-	-	-	-
Stage 2	391	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	117	475	930	-	-	-
Mov Cap-2 Maneuver	117	-	-	-	-	-
Stage 1	495	-	-	-	-	-
Stage 2	391	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v	27.12	0.68	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	930	-	117	475	-	-
HCM Lane V/C Ratio	0.069	-	0.392	0.211	-	-
HCM Control Delay (s/veh)	9.2	-	54.5	14.6	-	-
HCM Lane LOS	A	-	F	B	-	-
HCM 95th %tile Q(veh)	0.2	-	1.6	0.8	-	-

Intersection						
Int Delay, s/veh	3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↙	↗	↙	↗	↗	↗
Traffic Vol, veh/h	27	68	145	712	798	78
Future Vol, veh/h	27	68	145	712	798	78
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	195	-	-	145
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	7	0	0	2	3	2
Mvmt Flow	28	70	149	734	823	80

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1856	823	903	0	-	0
Stage 1	823	-	-	-	-	-
Stage 2	1033	-	-	-	-	-
Critical Hdwy	6.47	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	79	377	761	-	-	-
Stage 1	423	-	-	-	-	-
Stage 2	336	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	63	377	761	-	-	-
Mov Cap-2 Maneuver	63	-	-	-	-	-
Stage 1	340	-	-	-	-	-
Stage 2	336	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s/v40.64		1.84	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	761	-	63	377	-	-
HCM Lane V/C Ratio	0.196	-	0.441	0.186	-	-
HCM Control Delay (s/veh)	10.9	-	100.9	16.7	-	-
HCM Lane LOS	B	-	F	C	-	-
HCM 95th %tile Q(veh)	0.7	-	1.7	0.7	-	-

CR 700 N & CR 650 E

***TRAFFIC VOLUME COUNTS
CAPACITY ANALYSIS***

CR 700 N & CR 650 E - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142921, Location: 39.865393, -86.41116



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	North Southbound				West Eastbound				East Westbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
2023-12-12 7:30AM	33	0	0	33	1	0	0	1	0	15	0	15	49
7:45AM	39	0	0	39	0	0	0	0	0	36	0	36	75
8:00AM	67	0	0	67	1	0	0	1	0	58	0	58	126
8:15AM	106	0	0	106	0	0	0	0	0	48	0	48	154
Total	245	0	0	245	2	0	0	2	0	157	0	157	404
% Approach	100%	0%	0%	-	100%	0%	0%	-	0%	100%	0%	-	-
% Total	60.6%	0%	0%	60.6%	0.5%	0%	0%	0.5%	0%	38.9%	0%	38.9%	-
PHF	0.578	-	-	0.578	0.500	-	-	0.500	-	0.677	-	0.677	0.656
Lights and Motorcycles	238	0	0	238	2	0	0	2	0	153	0	153	393
% Lights and Motorcycles	97.1%	0%	0%	97.1%	100%	0%	0%	100%	0%	97.5%	0%	97.5%	97.3%
Heavy	7	0	0	7	0	0	0	0	0	4	0	4	11
% Heavy	2.9%	0%	0%	2.9%	0%	0%	0%	0%	0%	2.5%	0%	2.5%	2.7%

*L: Left, R: Right, T: Thru, U: U-Turn

CR 700 N & CR 650 E - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

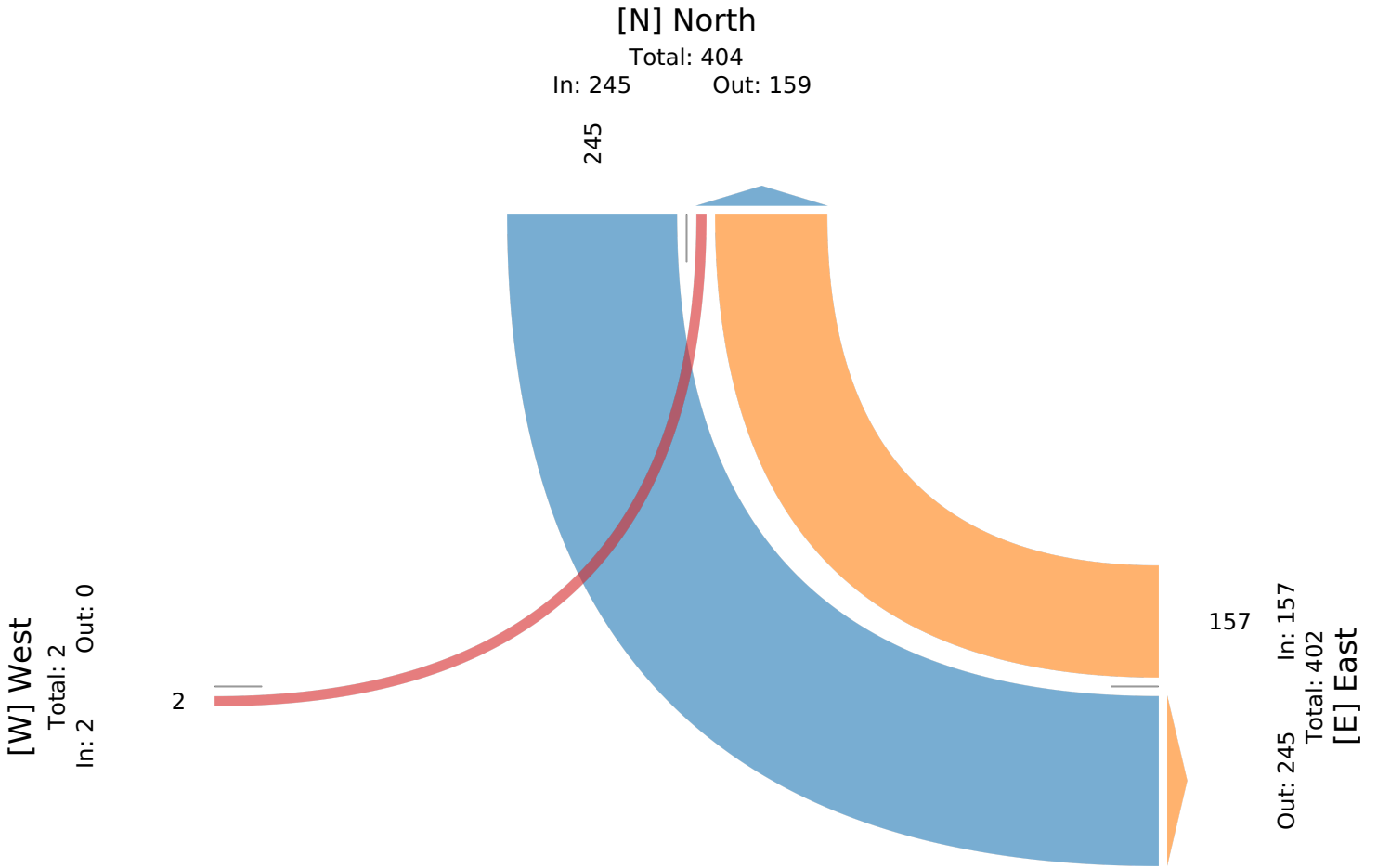
All Movements

ID: 1142921, Location: 39.865393, -86.41116



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



CR 700 N & CR 650 E - TMC

Tue Dec 12, 2023

PM Peak (3:30 PM - 4:30 PM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142921, Location: 39.865393, -86.41116



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	North Southbound				West Eastbound				East Westbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
2023-12-12 3:30PM	49	0	0	49	1	0	0	1	0	44	0	44	94
3:45PM	71	0	0	71	0	0	0	0	0	26	0	26	97
4:00PM	27	1	1	29	0	0	0	0	0	37	0	37	66
4:15PM	29	0	0	29	0	0	0	0	2	43	0	45	74
Total	176	1	1	178	1	0	0	1	2	150	0	152	331
% Approach	98.9%	0.6%	0.6%	-	100%	0%	0%	-	1.3%	98.7%	0%	-	-
% Total	53.2%	0.3%	0.3%	53.8%	0.3%	0%	0%	0.3%	0.6%	45.3%	0%	45.9%	-
PHF	0.620	0.250	0.250	0.627	0.250	-	-	0.250	0.250	0.852	-	0.844	0.853
Lights and Motorcycles	167	0	0	167	0	0	0	0	2	148	0	150	317
% Lights and Motorcycles	94.9%	0%	0%	93.8%	0%	0%	0%	0%	100%	98.7%	0%	98.7%	95.8%
Heavy	9	1	1	11	1	0	0	1	0	2	0	2	14
% Heavy	5.1%	100%	100%	6.2%	100%	0%	0%	100%	0%	1.3%	0%	1.3%	4.2%

*L: Left, R: Right, T: Thru, U: U-Turn

CR 700 N & CR 650 E - TMC

Tue Dec 12, 2023

PM Peak (3:30 PM - 4:30 PM)

All Classes (Lights and Motorcycles, Heavy)

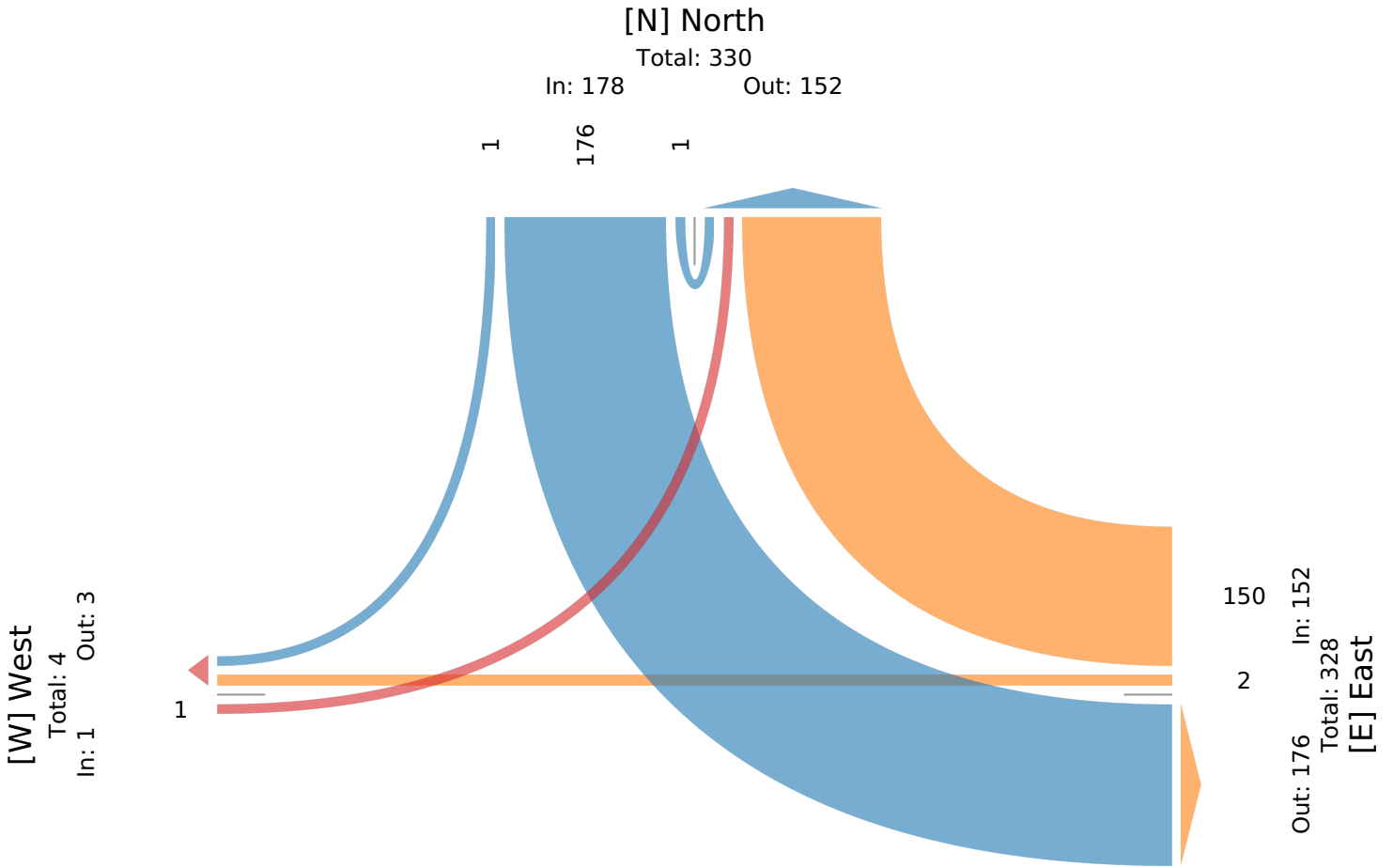
All Movements

ID: 1142921, Location: 39.865393, -86.41116



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



Intersection						
Int Delay, s/veh	7.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	2	0	0	157	245	0
Future Vol, veh/h	2	0	0	157	245	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	66	66	66	66	66	66
Heavy Vehicles, %	0	0	0	3	3	0
Mvmt Flow	3	0	0	238	371	0

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	238	0	-	0	125
Stage 1	-	-	-	-	119
Stage 2	-	-	-	-	6
Critical Hdwy	4.1	-	-	-	6.43
Critical Hdwy Stg 1	-	-	-	-	5.43
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	2.2	-	-	-	3.527
Pot Cap-1 Maneuver	1341	-	-	-	867
Stage 1	-	-	-	-	904
Stage 2	-	-	-	-	1014
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1341	-	-	-	866
Mov Cap-2 Maneuver	-	-	-	-	866
Stage 1	-	-	-	-	902
Stage 2	-	-	-	-	1014

Approach	EB	WB	SB
HCM Control Delay, s/v	7.69	0	12.25
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1341	-	-	-	866
HCM Lane V/C Ratio	0.002	-	-	-	0.429
HCM Control Delay (s/veh)	7.7	0	-	-	12.2
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	2.2

Intersection						
Int Delay, s/veh	5.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	1	0	2	150	176	1
Future Vol, veh/h	1	0	2	150	176	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	100	0	0	1	5	100
Mvmt Flow	1	0	2	176	207	1

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	179	0	-	0	93
Stage 1	-	-	-	-	91
Stage 2	-	-	-	-	2
Critical Hdwy	5.1	-	-	-	6.45
Critical Hdwy Stg 1	-	-	-	-	5.45
Critical Hdwy Stg 2	-	-	-	-	5.45
Follow-up Hdwy	3.1	-	-	-	3.545
Pot Cap-1 Maneuver	973	-	-	-	900
Stage 1	-	-	-	-	925
Stage 2	-	-	-	-	1013
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	973	-	-	-	899
Mov Cap-2 Maneuver	-	-	-	-	899
Stage 1	-	-	-	-	924
Stage 2	-	-	-	-	1013

Approach	EB	WB	SB
HCM Control Delay, s/v	8.71	0	10.22
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	973	-	-	-	898
HCM Lane V/C Ratio	0.001	-	-	-	0.232
HCM Control Delay (s/veh)	8.7	0	-	-	10.2
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.9

Intersection						
Int Delay, s/veh	8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↶	↷		↶	↷
Traffic Vol, veh/h	3	0	0	172	268	0
Future Vol, veh/h	3	0	0	172	268	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	66	66	66	66	66	66
Heavy Vehicles, %	0	0	0	3	3	0
Mvmt Flow	5	0	0	261	406	0

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	261	0	-	0	139
Stage 1	-	-	-	-	130
Stage 2	-	-	-	-	9
Critical Hdwy	4.1	-	-	-	6.43
Critical Hdwy Stg 1	-	-	-	-	5.43
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	2.2	-	-	-	3.527
Pot Cap-1 Maneuver	1316	-	-	-	851
Stage 1	-	-	-	-	893
Stage 2	-	-	-	-	1011
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1316	-	-	-	848
Mov Cap-2 Maneuver	-	-	-	-	848
Stage 1	-	-	-	-	890
Stage 2	-	-	-	-	1011

Approach	EB	WB	SB
HCM Control Delay, s/v	7.75	0	13.08
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1316	-	-	-	848
HCM Lane V/C Ratio	0.003	-	-	-	0.479
HCM Control Delay (s/veh)	7.7	0	-	-	13.1
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	2.6

Intersection						
Int Delay, s/veh	5.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	2	0	3	164	192	2
Future Vol, veh/h	2	0	3	164	192	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	100	0	0	1	5	100
Mvmt Flow	2	0	4	193	226	2

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	196	0	-	0	105
Stage 1	-	-	-	-	100
Stage 2	-	-	-	-	5
Critical Hdwy	5.1	-	-	-	6.45
Critical Hdwy Stg 1	-	-	-	-	5.45
Critical Hdwy Stg 2	-	-	-	-	5.45
Follow-up Hdwy	3.1	-	-	-	3.545
Pot Cap-1 Maneuver	956	-	-	-	886
Stage 1	-	-	-	-	917
Stage 2	-	-	-	-	1011
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	956	-	-	-	884
Mov Cap-2 Maneuver	-	-	-	-	884
Stage 1	-	-	-	-	914
Stage 2	-	-	-	-	1011

Approach	EB	WB	SB
HCM Control Delay, s/v	8.78	0	10.5
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	956	-	-	-	882
HCM Lane V/C Ratio	0.002	-	-	-	0.259
HCM Control Delay (s/veh)	8.8	0	-	-	10.5
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	1

Intersection						
Int Delay, s/veh	8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	3	0	0	172	268	0
Future Vol, veh/h	3	0	0	172	268	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	66	66	66	66	66	66
Heavy Vehicles, %	0	0	0	3	3	0
Mvmt Flow	5	0	0	261	406	0

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	261	0	-	0	139
Stage 1	-	-	-	-	130
Stage 2	-	-	-	-	9
Critical Hdwy	4.1	-	-	-	6.43
Critical Hdwy Stg 1	-	-	-	-	5.43
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	2.2	-	-	-	3.527
Pot Cap-1 Maneuver	1316	-	-	-	851
Stage 1	-	-	-	-	893
Stage 2	-	-	-	-	1011
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1316	-	-	-	848
Mov Cap-2 Maneuver	-	-	-	-	848
Stage 1	-	-	-	-	890
Stage 2	-	-	-	-	1011

Approach	EB	WB	SB
HCM Control Delay, s/v	7.75	0	13.08
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1316	-	-	-	848
HCM Lane V/C Ratio	0.003	-	-	-	0.479
HCM Control Delay (s/veh)	7.7	0	-	-	13.1
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	2.6

Intersection						
Int Delay, s/veh	5.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	2	0	3	164	192	2
Future Vol, veh/h	2	0	3	164	192	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	100	0	0	1	5	100
Mvmt Flow	2	0	4	193	226	2

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	196	0	-	0	105
Stage 1	-	-	-	-	100
Stage 2	-	-	-	-	5
Critical Hdwy	5.1	-	-	-	6.45
Critical Hdwy Stg 1	-	-	-	-	5.45
Critical Hdwy Stg 2	-	-	-	-	5.45
Follow-up Hdwy	3.1	-	-	-	3.545
Pot Cap-1 Maneuver	956	-	-	-	886
Stage 1	-	-	-	-	917
Stage 2	-	-	-	-	1011
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	956	-	-	-	884
Mov Cap-2 Maneuver	-	-	-	-	884
Stage 1	-	-	-	-	914
Stage 2	-	-	-	-	1011

Approach	EB	WB	SB
HCM Control Delay, s/v	8.78	0	10.5
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	956	-	-	-	882
HCM Lane V/C Ratio	0.002	-	-	-	0.259
HCM Control Delay (s/veh)	8.8	0	-	-	10.5
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	1

CR 700 N & SR 267

TRAFFIC VOLUME COUNTS CAPACITY ANALYSIS

SR 267 & CR 700 N - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142918, Location: 39.865527, -86.39247



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2023-12-12 7:30AM	45	170	16	0	231	7	180	2	0	189	10	5	61	0	76	57	5	13	0	75	571
7:45AM	67	185	15	0	267	5	173	0	0	178	7	4	60	0	71	37	5	8	0	50	566
8:00AM	61	147	20	0	228	9	148	5	0	162	11	2	82	0	95	54	5	8	0	67	552
8:15AM	56	142	21	0	219	8	178	5	0	191	6	3	129	0	138	61	4	13	1	79	627
Total	229	644	72	0	945	29	679	12	0	720	34	14	332	0	380	209	19	42	1	271	2316
% Approach	24.2%	68.1%	7.6%	0%	-	4.0%	94.3%	1.7%	0%	-	8.9%	3.7%	87.4%	0%	-	77.1%	7.0%	15.5%	0.4%	-	-
% Total	9.9%	27.8%	3.1%	0%	40.8%	1.3%	29.3%	0.5%	0%	31.1%	1.5%	0.6%	14.3%	0%	16.4%	9.0%	0.8%	1.8%	0%	11.7%	-
PHF	0.854	0.870	0.857	-	0.885	0.806	0.943	0.600	-	0.942	0.773	0.700	0.643	-	0.688	0.857	0.950	0.808	0.250	0.858	0.923
Lights and Motorcycles	225	623	67	0	915	27	661	12	0	700	34	14	325	0	373	209	18	41	1	269	2257
% Lights and Motorcycles	98.3%	96.7%	93.1%	0%	96.8%	93.1%	97.3%	100%	0%	97.2%	100%	100%	97.9%	0%	98.2%	100%	94.7%	97.6%	100%	99.3%	97.5%
Heavy	4	21	5	0	30	2	18	0	0	20	0	0	7	0	7	0	1	1	0	2	59
% Heavy	1.7%	3.3%	6.9%	0%	3.2%	6.9%	2.7%	0%	0%	2.8%	0%	0%	2.1%	0%	1.8%	0%	5.3%	2.4%	0%	0.7%	2.5%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & CR 700 N - TMC

Tue Dec 12, 2023

AM Peak (7:30 AM - 8:30 AM)

All Classes (Lights and Motorcycles, Heavy)

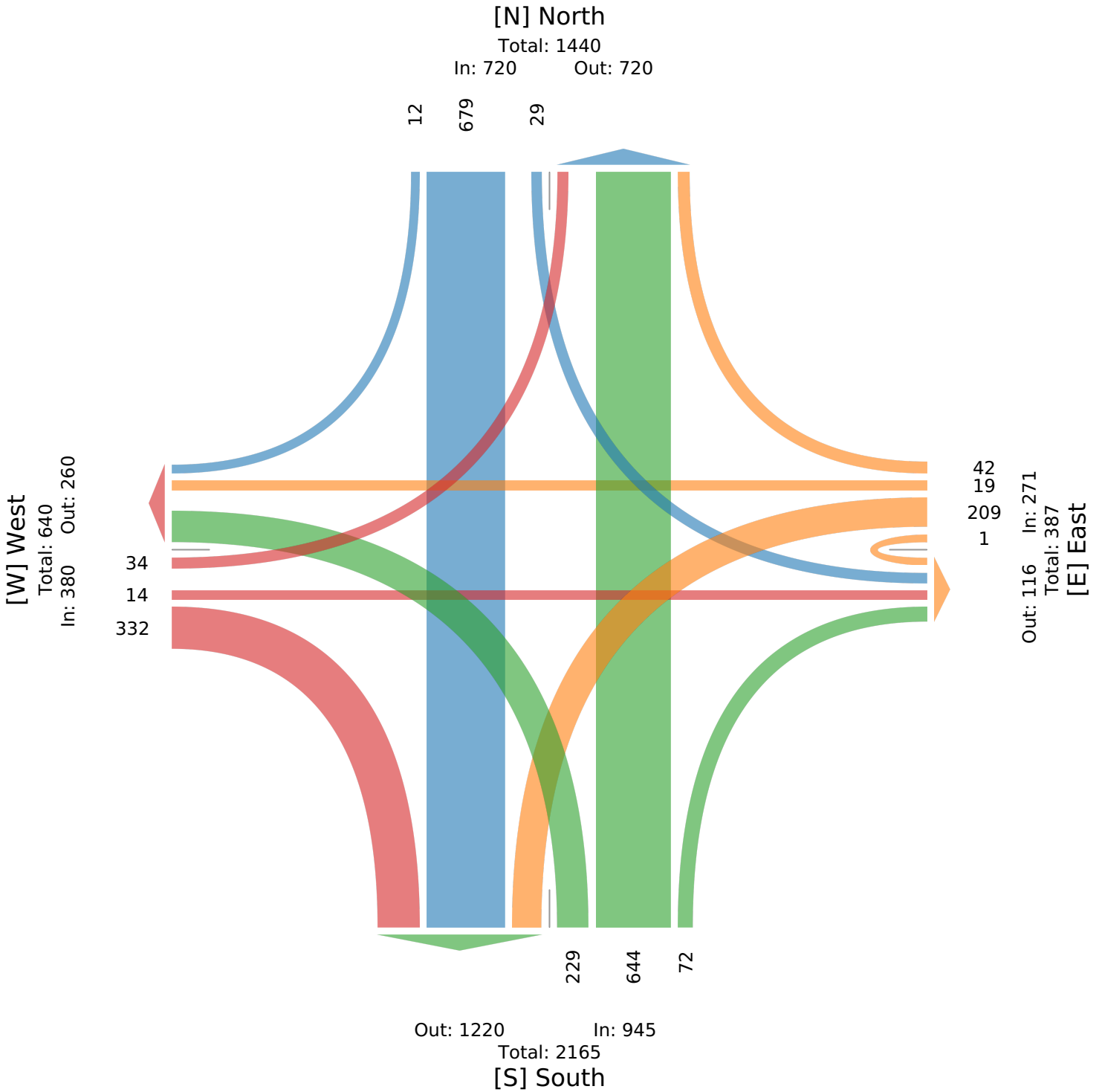
All Movements

ID: 1142918, Location: 39.865527, -86.39247



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



SR 267 & CR 700 N - TMC

Tue Dec 12, 2023

PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1142918, Location: 39.865527, -86.39247



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					
Time	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	Int
2023-12-12 4:45PM	63	215	42	1	321	26	191	0	0	217	4	2	46	0	52	78	9	13	0	100	690
5:00PM	78	228	36	0	342	23	132	2	0	157	4	12	33	1	50	86	8	14	0	108	657
5:15PM	52	225	40	0	317	24	194	5	0	223	4	4	55	0	63	70	8	15	0	93	696
5:30PM	64	225	49	0	338	21	167	0	0	188	3	10	58	0	71	81	9	19	0	109	706
Total	257	893	167	1	1318	94	684	7	0	785	15	28	192	1	236	315	34	61	0	410	2749
% Approach	19.5%	67.8%	12.7%	0.1%	-	12.0%	87.1%	0.9%	0%	-	6.4%	11.9%	81.4%	0.4%	-	76.8%	8.3%	14.9%	0%	-	-
% Total	9.3%	32.5%	6.1%	0%	47.9%	3.4%	24.9%	0.3%	0%	28.6%	0.5%	1.0%	7.0%	0%	8.6%	11.5%	1.2%	2.2%	0%	14.9%	-
PHF	0.824	0.979	0.852	0.250	0.963	0.904	0.881	0.350	-	0.880	0.938	0.583	0.828	0.250	0.831	0.916	0.944	0.803	-	0.940	0.973
Lights and Motorcycles	256	878	167	1	1302	94	660	7	0	761	15	28	190	1	234	314	34	60	0	408	2705
% Lights and Motorcycles	99.6%	98.3%	100%	100%	98.8%	100%	96.5%	100%	0%	96.9%	100%	100%	99.0%	100%	99.2%	99.7%	100%	98.4%	0%	99.5%	98.4%
Heavy	1	15	0	0	16	0	24	0	0	24	0	0	2	0	2	1	0	1	0	2	44
% Heavy	0.4%	1.7%	0%	0%	1.2%	0%	3.5%	0%	0%	3.1%	0%	0%	1.0%	0%	0.8%	0.3%	0%	1.6%	0%	0.5%	1.6%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & CR 700 N - TMC

Tue Dec 12, 2023

PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

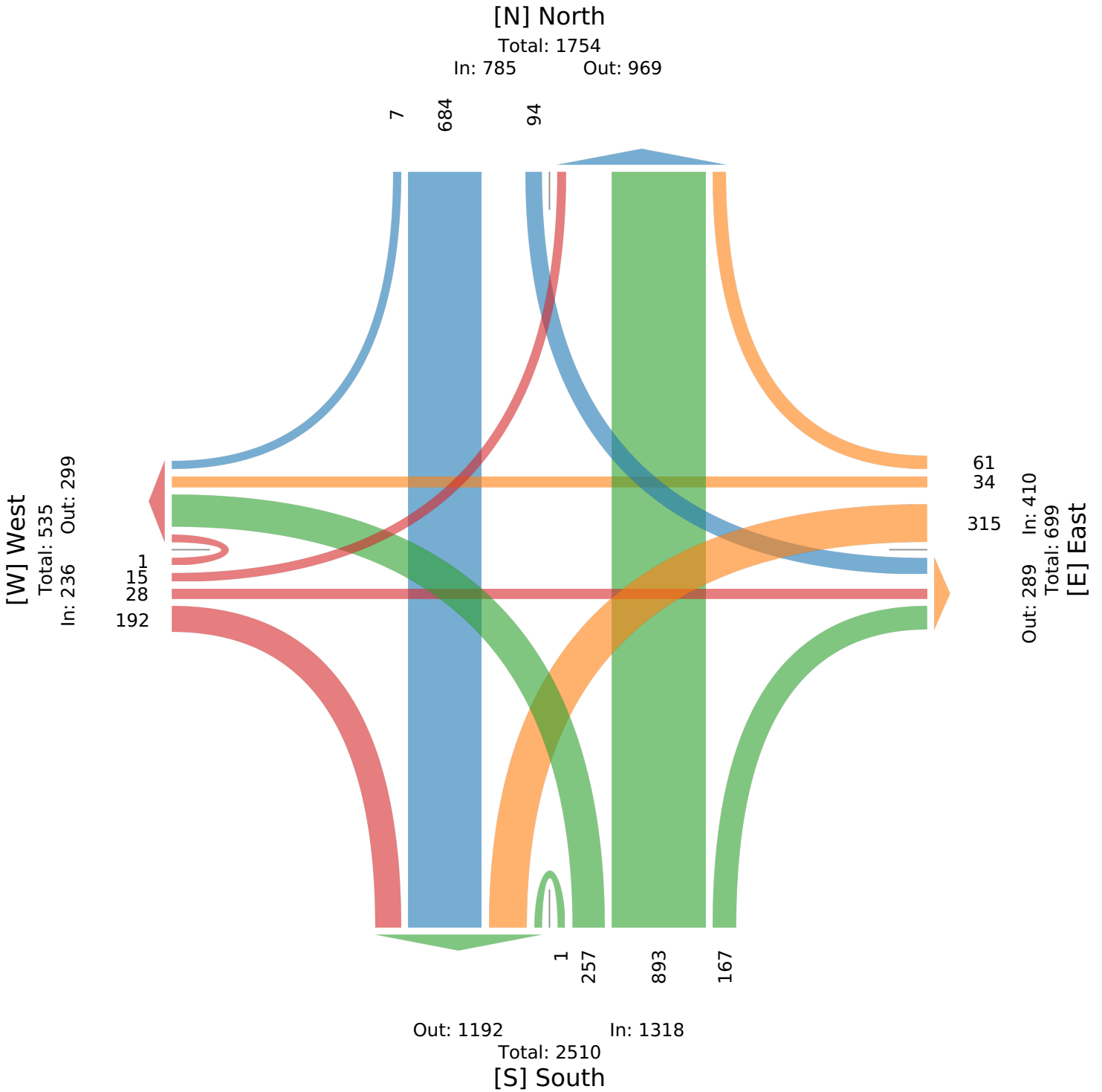
All Movements

ID: 1142918, Location: 39.865527, -86.39247



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

























HCM 7th Signalized Intersection Summary
4: SR 267 & CR 700 N

Existing AM Peak
07/11/2024

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	34	14	332	209	19	42	229	644	72	29	679	12
Future Volume (veh/h)	34	14	332	209	19	42	229	644	72	29	679	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1870	1900	1826	1870	1870	1856	1796	1796	1856	1900
Adj Flow Rate, veh/h	37	15	361	227	21	46	249	700	78	32	738	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	2	0	5	2	2	3	7	7	3	0
Cap, veh/h	458	353	499	374	126	276	403	1147	128	312	981	17
Arrive On Green	0.05	0.19	0.19	0.11	0.25	0.25	0.13	0.36	0.36	0.05	0.28	0.28
Sat Flow, veh/h	1810	1900	1585	3510	509	1116	1781	3198	356	1711	3545	62
Grp Volume(v), veh/h	37	15	361	227	0	67	249	386	392	32	367	384
Grp Sat Flow(s),veh/h/ln	1810	1900	1585	1755	0	1625	1781	1763	1791	1711	1763	1844
Q Serve(g_s), s	1.0	0.4	12.0	4.0	0.0	2.1	5.9	11.6	11.6	0.8	12.3	12.3
Cycle Q Clear(g_c), s	1.0	0.4	12.0	4.0	0.0	2.1	5.9	11.6	11.6	0.8	12.3	12.3
Prop In Lane	1.00		1.00	1.00		0.69	1.00		0.20	1.00		0.03
Lane Grp Cap(c), veh/h	458	353	499	374	0	402	403	632	643	312	488	511
V/C Ratio(X)	0.08	0.04	0.72	0.61	0.00	0.17	0.62	0.61	0.61	0.10	0.75	0.75
Avail Cap(c_a), veh/h	562	353	499	543	0	402	724	1129	1148	419	777	813
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.1	21.6	19.6	27.6	0.0	19.1	14.3	17.0	17.0	15.4	21.3	21.3
Incr Delay (d2), s/veh	0.1	0.0	5.1	1.6	0.0	0.2	1.5	1.0	0.9	0.1	2.4	2.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.2	5.1	1.7	0.0	0.8	2.1	4.1	4.2	0.3	4.7	4.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	19.2	21.6	24.8	29.2	0.0	19.3	15.8	18.0	18.0	15.5	23.7	23.6
LnGrp LOS	B	C	C	C		B	B	B	B	B	C	C
Approach Vol, veh/h		413			294			1027			783	
Approach Delay, s/veh		24.2			26.9			17.4			23.3	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.6	28.2	11.9	17.0	12.8	22.9	7.9	21.0				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	7.1	41.4	10.0	12.0	20.0	28.5	7.1	15.4				
Max Q Clear Time (g_c+I1), s	2.8	13.6	6.0	14.0	7.9	14.3	3.0	4.1				
Green Ext Time (p_c), s	0.0	4.7	0.3	0.0	0.5	3.6	0.0	0.2				
Intersection Summary												
HCM 7th Control Delay, s/veh			21.5									
HCM 7th LOS			C									






















HCM 7th Signalized Intersection Summary
 4: SR 267 & CR 700 N

Existing PM Peak
 07/11/2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	15	28	192	315	34	61	257	893	167	94	684	7
Future Volume (veh/h)	15	28	192	315	34	61	257	893	167	94	684	7
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1885	1900	1900	1870	1900	1870	1900	1900	1841	1900
Adj Flow Rate, veh/h	15	29	198	325	35	63	265	921	172	97	705	7
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	0	1	0	0	2	0	2	0	0	4	0
Cap, veh/h	340	292	440	438	158	285	453	1172	219	299	1239	12
Arrive On Green	0.02	0.15	0.15	0.12	0.26	0.26	0.12	0.39	0.39	0.08	0.35	0.35
Sat Flow, veh/h	1810	1900	1598	3510	608	1095	1810	2989	558	1810	3548	35
Grp Volume(v), veh/h	15	29	198	325	0	98	265	547	546	97	347	365
Grp Sat Flow(s),veh/h/ln	1810	1900	1598	1755	0	1703	1810	1777	1770	1810	1749	1834
Q Serve(g_s), s	0.5	1.0	8.0	7.0	0.0	3.5	6.9	21.1	21.1	2.5	12.6	12.6
Cycle Q Clear(g_c), s	0.5	1.0	8.0	7.0	0.0	3.5	6.9	21.1	21.1	2.5	12.6	12.6
Prop In Lane	1.00		1.00	1.00		0.64	1.00		0.32	1.00		0.02
Lane Grp Cap(c), veh/h	340	292	440	438	0	443	453	697	694	299	611	641
V/C Ratio(X)	0.04	0.10	0.45	0.74	0.00	0.22	0.58	0.79	0.79	0.32	0.57	0.57
Avail Cap(c_a), veh/h	469	293	440	811	0	503	733	1159	1154	381	875	918
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	26.5	28.3	23.3	32.9	0.0	22.6	14.0	20.8	20.8	15.9	20.6	20.6
Incr Delay (d2), s/veh	0.1	0.1	0.7	2.5	0.0	0.2	1.2	2.0	2.0	0.6	0.8	0.8
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.5	3.0	3.0	0.0	1.4	2.5	8.0	8.0	0.9	4.7	4.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	26.5	28.5	24.1	35.4	0.0	22.9	15.2	22.8	22.8	16.6	21.4	21.4
LnGrp LOS	C	C	C	D		C	B	C	C	B	C	C
Approach Vol, veh/h		242			423			1358			809	
Approach Delay, s/veh		24.7			32.5			21.3			20.8	
Approach LOS		C			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.6	35.5	14.7	17.0	14.0	32.2	6.4	25.3				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	9.7	50.8	18.0	12.0	21.5	39.0	7.5	23.0				
Max Q Clear Time (g_c+I1), s	4.5	23.1	9.0	10.0	8.9	14.6	2.5	5.5				
Green Ext Time (p_c), s	0.1	7.4	0.8	0.2	0.6	4.1	0.0	0.4				
Intersection Summary												
HCM 7th Control Delay, s/veh			23.1									
HCM 7th LOS			C									

HCM 7th Signalized Intersection Summary
4: SR 267 & CR 700 N

Background 2029 AM Peak
07/11/2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	38	16	362	235	22	48	271	760	85	31	720	13
Future Volume (veh/h)	38	16	362	235	22	48	271	760	85	31	720	13
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1870	1900	1826	1870	1870	1856	1796	1796	1856	1900
Adj Flow Rate, veh/h	41	17	393	255	24	52	295	826	92	34	783	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	2	0	5	2	2	3	7	7	3	0
Cap, veh/h	445	336	510	359	119	257	423	1229	137	289	1022	18
Arrive On Green	0.06	0.18	0.18	0.10	0.23	0.23	0.14	0.38	0.38	0.05	0.29	0.29
Sat Flow, veh/h	1810	1900	1585	3510	513	1112	1781	3198	356	1711	3544	63
Grp Volume(v), veh/h	41	17	393	255	0	76	295	455	463	34	389	408
Grp Sat Flow(s),veh/h/ln	1810	1900	1585	1755	0	1626	1781	1763	1791	1711	1763	1844
Q Serve(g_s), s	1.2	0.5	12.0	4.8	0.0	2.6	7.2	14.5	14.5	0.9	13.7	13.7
Cycle Q Clear(g_c), s	1.2	0.5	12.0	4.8	0.0	2.6	7.2	14.5	14.5	0.9	13.7	13.7
Prop In Lane	1.00		1.00	1.00		0.68	1.00		0.20	1.00		0.03
Lane Grp Cap(c), veh/h	445	336	510	359	0	376	423	677	688	289	509	532
V/C Ratio(X)	0.09	0.05	0.77	0.71	0.00	0.20	0.70	0.67	0.67	0.12	0.77	0.77
Avail Cap(c_a), veh/h	534	336	510	466	0	376	678	1102	1120	385	780	816
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.4	23.2	20.8	29.5	0.0	21.0	14.6	17.3	17.3	15.7	22.0	22.0
Incr Delay (d2), s/veh	0.1	0.1	7.1	3.5	0.0	0.3	2.1	1.2	1.1	0.2	2.4	2.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.2	6.2	2.1	0.0	1.0	2.6	5.1	5.2	0.3	5.3	5.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	20.5	23.2	27.9	32.9	0.0	21.3	16.7	18.5	18.5	15.9	24.5	24.4
LnGrp LOS	C	C	C	C		C	B	B	B	B	C	C
Approach Vol, veh/h		451			331			1213			831	
Approach Delay, s/veh		27.0			30.2			18.0			24.1	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.8	31.1	11.9	17.0	14.3	24.6	8.3	20.7				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	7.1	42.4	9.0	12.0	19.5	30.0	7.1	14.4				
Max Q Clear Time (g_c+I1), s	2.9	16.5	6.8	14.0	9.2	15.7	3.2	4.6				
Green Ext Time (p_c), s	0.0	5.7	0.2	0.0	0.6	3.9	0.0	0.2				
Intersection Summary												
HCM 7th Control Delay, s/veh			22.7									
HCM 7th LOS			C									

HCM 7th Signalized Intersection Summary
4: SR 267 & CR 700 N

Background 2029 PM Peak
07/11/2024

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	31	210	353	39	69	304	1054	198	100	726	8
Future Volume (veh/h)	17	31	210	353	39	69	304	1054	198	100	726	8
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1885	1900	1900	1870	1900	1870	1900	1900	1841	1900
Adj Flow Rate, veh/h	18	32	216	364	40	71	313	1087	204	103	748	8
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	0	1	0	0	2	0	2	0	0	4	0
Cap, veh/h	308	257	421	462	150	266	472	1316	246	264	1366	15
Arrive On Green	0.03	0.14	0.14	0.13	0.24	0.24	0.13	0.44	0.44	0.07	0.39	0.39
Sat Flow, veh/h	1810	1900	1598	3510	614	1090	1810	2988	559	1810	3545	38
Grp Volume(v), veh/h	18	32	216	364	0	111	313	645	646	103	369	387
Grp Sat Flow(s),veh/h/ln	1810	1900	1598	1755	0	1704	1810	1777	1770	1810	1749	1834
Q Serve(g_s), s	0.7	1.3	10.2	8.9	0.0	4.7	8.6	28.3	28.5	2.9	14.6	14.6
Cycle Q Clear(g_c), s	0.7	1.3	10.2	8.9	0.0	4.7	8.6	28.3	28.5	2.9	14.6	14.6
Prop In Lane	1.00		1.00	1.00		0.64	1.00		0.32	1.00		0.02
Lane Grp Cap(c), veh/h	308	257	421	462	0	416	472	783	780	264	674	707
V/C Ratio(X)	0.06	0.12	0.51	0.79	0.00	0.27	0.66	0.82	0.83	0.39	0.55	0.55
Avail Cap(c_a), veh/h	402	257	421	712	0	449	740	1041	1037	306	710	744
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	31.4	33.7	27.8	37.3	0.0	27.1	14.3	21.8	21.9	17.9	21.2	21.2
Incr Delay (d2), s/veh	0.1	0.2	1.1	3.3	0.0	0.3	1.6	4.1	4.3	0.9	0.8	0.8
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.6	3.9	4.0	0.0	1.9	3.2	11.3	11.3	1.1	5.6	5.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	31.4	34.0	28.9	40.6	0.0	27.4	15.9	25.9	26.2	18.9	22.0	22.0
LnGrp LOS	C	C	C	D		C	B	C	C	B	C	C
Approach Vol, veh/h		266			475			1604			859	
Approach Delay, s/veh		29.7			37.5			24.1			21.6	
Approach LOS		C			D			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.9	44.1	16.7	17.0	15.9	39.2	7.0	26.7				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	8.5	52.0	18.0	12.0	24.5	36.0	7.1	23.4				
Max Q Clear Time (g_c+I1), s	4.9	30.5	10.9	12.2	10.6	16.6	2.7	6.7				
Green Ext Time (p_c), s	0.1	8.6	0.8	0.0	0.7	4.1	0.0	0.5				
Intersection Summary												
HCM 7th Control Delay, s/veh				25.9								
HCM 7th LOS				C								

HCM 7th Signalized Intersection Summary
4: SR 267 & CR 700 N

Background 2029 + Proposed AM Peak























07/11/2024

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	38	16	362	235	22	50	271	772	85	37	755	13
Future Volume (veh/h)	38	16	362	235	22	50	271	772	85	37	755	13
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1870	1900	1826	1870	1870	1856	1796	1796	1856	1900
Adj Flow Rate, veh/h	41	17	393	255	24	54	295	839	92	40	821	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	2	0	5	2	2	3	7	7	3	0
Cap, veh/h	438	331	501	356	114	256	416	1241	136	295	1063	18
Arrive On Green	0.06	0.17	0.17	0.10	0.23	0.23	0.14	0.39	0.39	0.05	0.30	0.30
Sat Flow, veh/h	1810	1900	1585	3510	500	1124	1781	3204	351	1711	3547	60
Grp Volume(v), veh/h	41	17	393	255	0	78	295	462	469	40	408	427
Grp Sat Flow(s),veh/h/ln	1810	1900	1585	1755	0	1624	1781	1763	1792	1711	1763	1845
Q Serve(g_s), s	1.2	0.5	12.0	4.9	0.0	2.7	7.2	15.0	15.0	1.1	14.5	14.5
Cycle Q Clear(g_c), s	1.2	0.5	12.0	4.9	0.0	2.7	7.2	15.0	15.0	1.1	14.5	14.5
Prop In Lane	1.00		1.00	1.00		0.69	1.00		0.20	1.00		0.03
Lane Grp Cap(c), veh/h	438	331	501	356	0	369	416	683	694	295	528	553
V/C Ratio(X)	0.09	0.05	0.78	0.72	0.00	0.21	0.71	0.68	0.68	0.14	0.77	0.77
Avail Cap(c_a), veh/h	525	331	501	458	0	369	641	1084	1103	379	793	830
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.9	23.7	21.4	30.0	0.0	21.6	14.7	17.5	17.5	15.4	22.0	22.0
Incr Delay (d2), s/veh	0.1	0.1	8.0	3.8	0.0	0.3	2.2	1.2	1.2	0.2	2.7	2.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.2	6.4	2.2	0.0	1.0	2.6	5.3	5.4	0.4	5.6	5.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	21.0	23.8	29.4	33.8	0.0	21.9	16.9	18.7	18.7	15.6	24.7	24.6
LnGrp LOS	C	C	C	C		C	B	B	B	B	C	C
Approach Vol, veh/h		451			333			1226			875	
Approach Delay, s/veh		28.5			31.0			18.3			24.2	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.2	31.7	12.0	17.0	14.3	25.6	8.3	20.7				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	7.1	42.4	9.0	12.0	18.5	31.0	7.1	14.4				
Max Q Clear Time (g_c+I1), s	3.1	17.0	6.9	14.0	9.2	16.5	3.2	4.7				
Green Ext Time (p_c), s	0.0	5.8	0.2	0.0	0.6	4.1	0.0	0.2				
Intersection Summary												
HCM 7th Control Delay, s/veh				23.1								
HCM 7th LOS				C								

HCM 7th Signalized Intersection Summary
4: SR 267 & CR 700 N

Background 2029 + Proposed PM Peak

07/11/2024

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	31	210	353	39	76	304	1093	198	104	748	8
Future Volume (veh/h)	17	31	210	353	39	76	304	1093	198	104	748	8
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1885	1900	1900	1870	1900	1870	1900	1900	1841	1900
Adj Flow Rate, veh/h	18	32	216	364	40	78	313	1127	204	107	771	8
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	0	1	0	0	2	0	2	0	0	4	0
Cap, veh/h	302	252	412	454	138	269	468	1356	244	260	1409	15
Arrive On Green	0.03	0.13	0.13	0.13	0.24	0.24	0.13	0.45	0.45	0.07	0.40	0.40
Sat Flow, veh/h	1810	1900	1598	3510	576	1122	1810	3008	542	1810	3546	37
Grp Volume(v), veh/h	18	32	216	364	0	118	313	664	667	107	380	399
Grp Sat Flow(s),veh/h/ln	1810	1900	1598	1755	0	1698	1810	1777	1773	1810	1749	1834
Q Serve(g_s), s	0.8	1.3	10.5	9.1	0.0	5.1	8.6	29.7	30.0	3.0	15.2	15.2
Cycle Q Clear(g_c), s	0.8	1.3	10.5	9.1	0.0	5.1	8.6	29.7	30.0	3.0	15.2	15.2
Prop In Lane	1.00		1.00	1.00		0.66	1.00		0.31	1.00		0.02
Lane Grp Cap(c), veh/h	302	252	412	454	0	406	468	801	799	260	695	729
V/C Ratio(X)	0.06	0.13	0.52	0.80	0.00	0.29	0.67	0.83	0.83	0.41	0.55	0.55
Avail Cap(c_a), veh/h	393	252	412	620	0	406	723	1040	1037	319	741	778
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.2	34.7	28.8	38.3	0.0	28.2	14.3	21.8	21.9	18.3	21.0	21.0
Incr Delay (d2), s/veh	0.1	0.2	1.2	5.3	0.0	0.4	1.7	4.5	4.7	1.0	0.7	0.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.6	4.1	4.2	0.0	2.1	3.2	11.9	12.0	1.2	5.8	6.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	32.3	34.9	30.1	43.6	0.0	28.6	15.9	26.3	26.6	19.3	21.7	21.7
LnGrp LOS	C	C	C	D		C	B	C	C	B	C	C
Approach Vol, veh/h		266			482			1644			886	
Approach Delay, s/veh		30.8			39.9			24.4			21.4	
Approach LOS		C			D			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.0	45.8	16.7	17.0	15.9	41.0	7.0	26.7				
Change Period (Y+Rc), s	4.5	5.0	5.0	5.0	4.5	5.0	4.5	5.0				
Max Green Setting (Gmax), s	9.5	53.0	16.0	12.0	24.1	38.4	7.1	21.4				
Max Q Clear Time (g_c+I1), s	5.0	32.0	11.1	12.5	10.6	17.2	2.8	7.1				
Green Ext Time (p_c), s	0.1	8.8	0.6	0.0	0.7	4.4	0.0	0.5				
Intersection Summary												
HCM 7th Control Delay, s/veh			26.4									
HCM 7th LOS			C									

***SR 267 (GREEN STREET)/ACRE LANE &
PROPOSED DEVELOPMENT ACCESS DRIVE***

***TRAFFIC VOLUME COUNTS
CAPACITY ANALYSIS***

SR 267 & ACRE LANE - TMC

Tue May 21, 2024

AM Peak (7:15 AM - 8:15 AM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1189708, Location: 39.875959, -86.392414



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2024-05-21 7:15AM	0	150	1	0	151	1	197	2	0	200	2	0	0	0	2	6	0	4	0	10	363
7:30AM	0	186	2	0	188	1	164	5	0	170	4	0	0	0	4	2	0	8	0	10	372
7:45AM	1	166	0	0	167	1	195	8	0	204	3	0	0	0	3	4	0	9	0	13	387
8:00AM	0	164	1	0	165	2	150	3	0	155	2	0	0	0	2	4	0	2	0	6	328
Total	1	666	4	0	671	5	706	18	0	729	11	0	0	0	11	16	0	23	0	39	1450
% Approach	0.1%	99.3%	0.6%	0%	-	0.7%	96.8%	2.5%	0%	-	100%	0%	0%	0%	-	41.0%	0%	59.0%	0%	-	-
% Total	0.1%	45.9%	0.3%	0%	46.3%	0.3%	48.7%	1.2%	0%	50.3%	0.8%	0%	0%	0%	0.8%	1.1%	0%	1.6%	0%	2.7%	-
PHF	0.250	0.895	0.500	-	0.892	0.625	0.896	0.563	-	0.893	0.688	-	-	-	0.688	0.667	-	0.639	-	0.750	0.937
Lights and Motorcycles	1	638	3	0	642	5	678	18	0	701	11	0	0	0	11	16	0	23	0	39	1393
% Lights and Motorcycles	100%	95.8%	75.0%	0%	95.7%	100%	96.0%	100%	0%	96.2%	100%	0%	0%	0%	100%	100%	0%	100%	0%	100%	96.1%
Heavy	0	28	1	0	29	0	28	0	0	28	0	0	0	0	0	0	0	0	0	0	57
% Heavy	0%	4.2%	25.0%	0%	4.3%	0%	4.0%	0%	0%	3.8%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3.9%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & ACRE LANE - TMC

Tue May 21, 2024

AM Peak (7:15 AM - 8:15 AM)

All Classes (Lights and Motorcycles, Heavy)

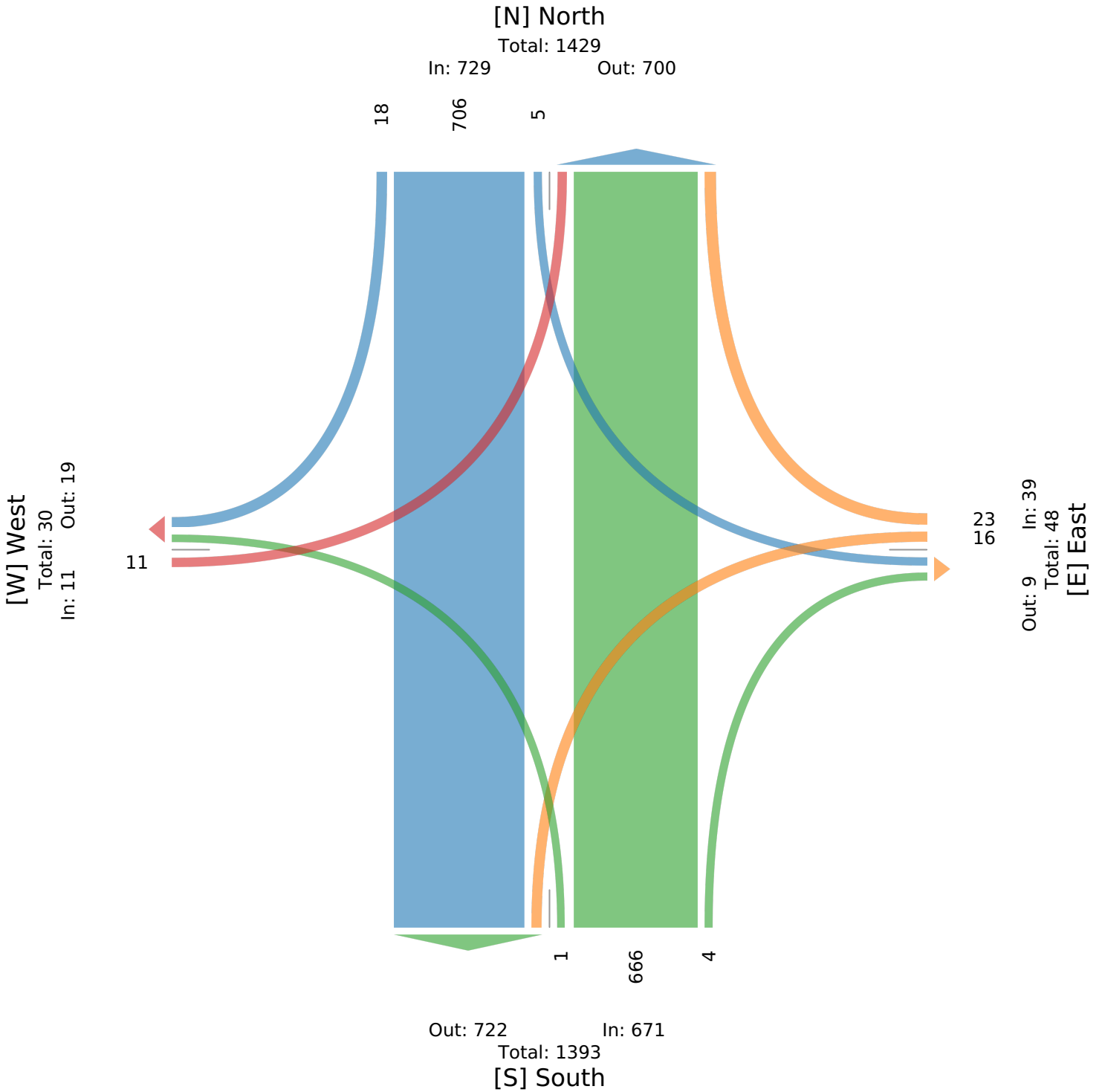
All Movements

ID: 1189708, Location: 39.875959, -86.392414



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



SR 267 & ACRE LANE - TMC

Tue May 21, 2024

PM Peak (4:15 PM - 5:15 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1189708, Location: 39.875959, -86.392414



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US

Leg Direction	South Northbound					North Southbound					West Eastbound					East Westbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2024-05-21 4:15PM	0	179	5	0	184	3	204	2	0	209	8	0	1	0	9	1	0	1	0	2	404
4:30PM	1	197	2	0	200	7	198	4	0	209	4	0	0	0	4	1	0	4	0	5	418
4:45PM	1	199	6	0	206	1	204	6	0	211	6	0	2	0	8	0	1	4	0	5	430
5:00PM	0	187	4	0	191	7	204	2	0	213	7	0	2	0	9	0	0	1	0	1	414
Total	2	762	17	0	781	18	810	14	0	842	25	0	5	0	30	2	1	10	0	13	1666
% Approach	0.3%	97.6%	2.2%	0%	-	2.1%	96.2%	1.7%	0%	-	83.3%	0%	16.7%	0%	-	15.4%	7.7%	76.9%	0%	-	-
% Total	0.1%	45.7%	1.0%	0%	46.9%	1.1%	48.6%	0.8%	0%	50.5%	1.5%	0%	0.3%	0%	1.8%	0.1%	0.1%	0.6%	0%	0.8%	-
PHF	0.500	0.957	0.708	-	0.948	0.643	0.993	0.583	-	0.988	0.781	-	0.625	-	0.833	0.500	0.250	0.625	-	0.650	0.969
Lights and Motorcycles	2	745	17	0	764	17	788	14	0	819	25	0	5	0	30	2	1	10	0	13	1626
% Lights and Motorcycles	100%	97.8%	100%	0%	97.8%	94.4%	97.3%	100%	0%	97.3%	100%	0%	100%	0%	100%	100%	100%	100%	0%	100%	97.6%
Heavy	0	17	0	0	17	1	22	0	0	23	0	0	0	0	0	0	0	0	0	0	40
% Heavy	0%	2.2%	0%	0%	2.2%	5.6%	2.7%	0%	0%	2.7%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2.4%

*L: Left, R: Right, T: Thru, U: U-Turn

SR 267 & ACRE LANE - TMC

Tue May 21, 2024

PM Peak (4:15 PM - 5:15 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

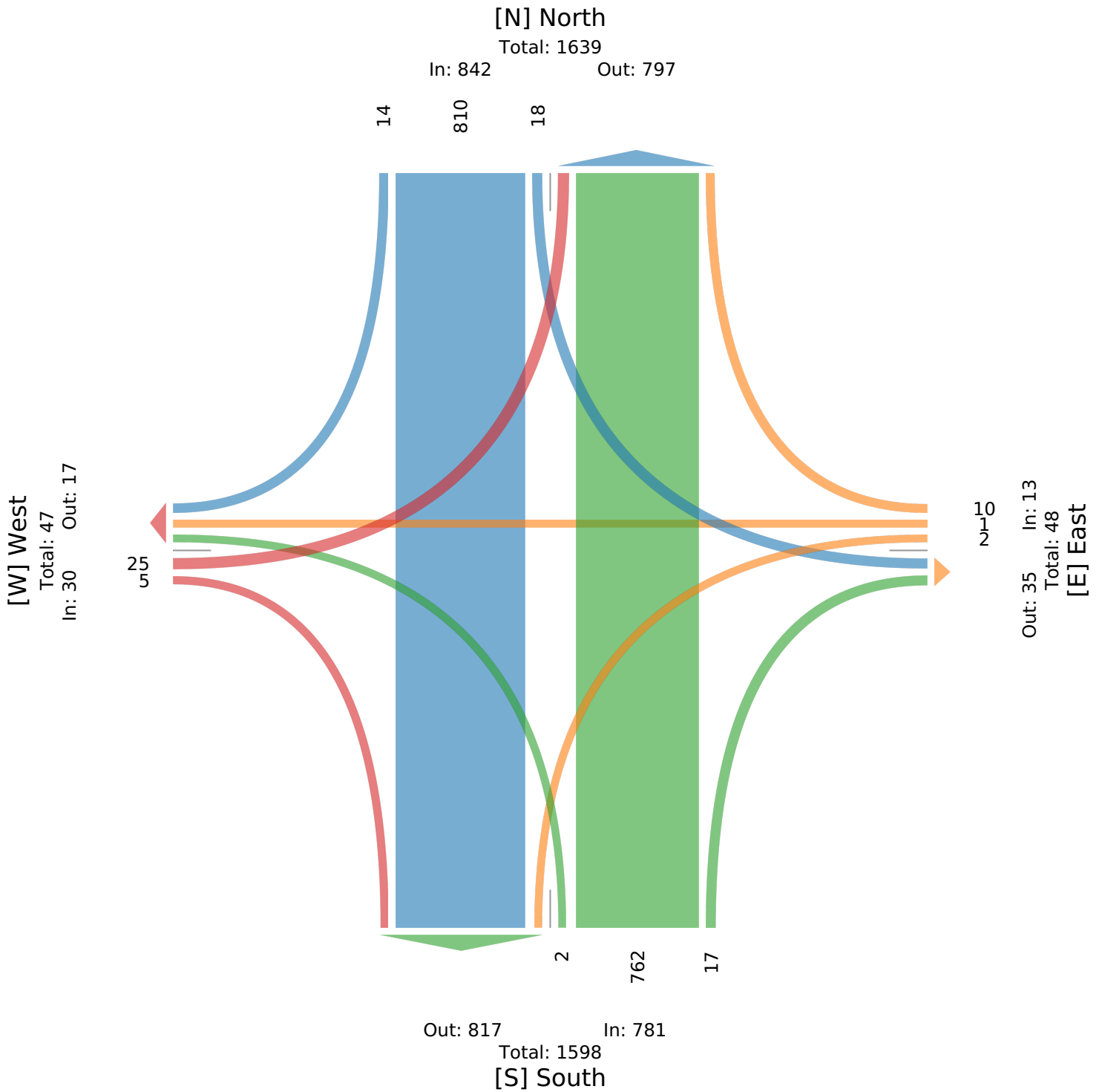
All Movements

ID: 1189708, Location: 39.875959, -86.392414



Provided by: A&F Engineering

8365 Keystone Crossing, Suite 201, Indianapolis, IN, 46240, US



Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕		↕	↗		↕	↗	
Traffic Vol, veh/h	11	0	0	16	0	23	1	666	4	5	706	18
Future Vol, veh/h	11	0	0	16	0	23	1	666	4	5	706	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0	0	4	25	0	4	0
Mvmt Flow	12	0	0	17	0	24	1	709	4	5	751	19

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1482	1486	761	1474	1494	711	770	0	0	713	0	0
Stage 1	771	771	-	713	713	-	-	-	-	-	-	-
Stage 2	711	715	-	762	781	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	104	126	409	106	124	437	853	-	-	896	-	-
Stage 1	396	412	-	426	439	-	-	-	-	-	-	-
Stage 2	427	438	-	401	408	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	98	125	409	105	123	437	853	-	-	896	-	-
Mov Cap-2 Maneuver	98	125	-	105	123	-	-	-	-	-	-	-
Stage 1	393	410	-	426	438	-	-	-	-	-	-	-
Stage 2	403	437	-	398	406	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB		
HCM Control Delay, s/v	46.71		29.15		0.01		0.06		
HCM LOS	E		D						

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	853	-	-	98	-	190	896	-	-
HCM Lane V/C Ratio	0.001	-	-	0.12	-	0.218	0.006	-	-
HCM Control Delay (s/veh)	9.2	-	-	46.7	0	29.1	9	-	-
HCM Lane LOS	A	-	-	E	A	D	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.4	-	0.8	0	-	-

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↔		↔		↔	↔		↔	↔	
Traffic Vol, veh/h	25	0	5	2	1	10	2	762	17	18	810	14
Future Vol, veh/h	25	0	5	2	1	10	2	762	17	18	810	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	6	3	0
Mvmt Flow	26	0	5	2	1	10	2	786	18	19	835	14

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1670	1687	842	1671	1685	794	849	0	0	803	0	0
Stage 1	879	879	-	798	798	-	-	-	-	-	-	-
Stage 2	790	807	-	872	887	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	77	95	367	77	95	391	797	-	-	803	-	-
Stage 1	345	368	-	382	401	-	-	-	-	-	-	-
Stage 2	386	397	-	348	365	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	73	92	367	74	93	391	797	-	-	803	-	-
Mov Cap-2 Maneuver	73	92	-	74	93	-	-	-	-	-	-	-
Stage 1	337	359	-	381	400	-	-	-	-	-	-	-
Stage 2	374	396	-	335	357	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s/v69.03		23.77	0.02	0.2
HCM LOS	F	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	797	-	-	73	367	205	803	-	-
HCM Lane V/C Ratio	0.003	-	-	0.355	0.014	0.065	0.023	-	-
HCM Control Delay (s/veh)	9.5	-	-	79.8	14.9	23.8	9.6	-	-
HCM Lane LOS	A	-	-	F	B	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	1.3	0	0.2	0.1	-	-

Intersection												
Int Delay, s/veh	1.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕		↕	↗		↕	↗	
Traffic Vol, veh/h	11	0	0	16	0	23	2	700	5	6	742	19
Future Vol, veh/h	11	0	0	16	0	23	2	700	5	6	742	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0	0	4	25	0	4	0
Mvmt Flow	12	0	0	17	0	24	2	745	5	6	789	20

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	1561	1566	799	1554	1574	747	810	0	0	750	0	0
Stage 1	812	812	-	752	752	-	-	-	-	-	-	-
Stage 2	749	754	-	802	822	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	92	112	388	93	111	416	825	-	-	868	-	-
Stage 1	376	395	-	406	421	-	-	-	-	-	-	-
Stage 2	407	420	-	380	391	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	86	111	388	92	110	416	825	-	-	868	-	-
Mov Cap-2 Maneuver	86	111	-	92	110	-	-	-	-	-	-	-
Stage 1	373	392	-	405	420	-	-	-	-	-	-	-
Stage 2	382	419	-	378	388	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s/v53.53			32.8		0.03		0.07	
HCM LOS	F		D					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	825	-	-	86	-	170	868	-	-
HCM Lane V/C Ratio	0.003	-	-	0.137	-	0.243	0.007	-	-
HCM Control Delay (s/veh)	9.4	-	-	53.5	0	32.8	9.2	-	-
HCM Lane LOS	A	-	-	F	A	D	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.5	-	0.9	0	-	-

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕		↕	↗		↕	↗	
Traffic Vol, veh/h	25	0	5	2	1	10	3	801	18	19	851	15
Future Vol, veh/h	25	0	5	2	1	10	3	801	18	19	851	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	6	3	0
Mvmt Flow	26	0	5	2	1	10	3	826	19	20	877	15

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1757	1775	885	1758	1773	835	893	0	0	844	0	0
Stage 1	924	924	-	841	841	-	-	-	-	-	-	-
Stage 2	832	851	-	916	932	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	67	84	347	67	84	371	768	-	-	775	-	-
Stage 1	326	351	-	362	383	-	-	-	-	-	-	-
Stage 2	366	379	-	329	348	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	63	81	347	64	81	371	768	-	-	775	-	-
Mov Cap-2 Maneuver	63	81	-	64	81	-	-	-	-	-	-	-
Stage 1	317	342	-	361	382	-	-	-	-	-	-	-
Stage 2	354	378	-	316	339	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s/v	84.3	26.04	0.04	0.21
HCM LOS	F	D		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	768	-	-	63	347	184	775	-	-
HCM Lane V/C Ratio	0.004	-	-	0.412	0.015	0.073	0.025	-	-
HCM Control Delay (s/veh)	9.7	-	-	98.1	15.5	26	9.8	-	-
HCM Lane LOS	A	-	-	F	C	D	A	-	-
HCM 95th %tile Q(veh)	0	-	-	1.6	0	0.2	0.1	-	-

Intersection												
Int Delay, s/veh	1.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕		↖	↗		↖	↗	
Traffic Vol, veh/h	17	0	10	16	0	23	5	711	5	6	773	21
Future Vol, veh/h	17	0	10	16	0	23	5	711	5	6	773	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0	0	4	25	0	4	0
Mvmt Flow	18	0	11	17	0	24	5	756	5	6	822	22

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1613	1619	834	1605	1627	759	845	0	0	762	0	0
Stage 1	846	846	-	770	770	-	-	-	-	-	-	-
Stage 2	767	772	-	835	857	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	85	104	371	86	103	410	801	-	-	860	-	-
Stage 1	360	381	-	396	413	-	-	-	-	-	-	-
Stage 2	398	412	-	365	377	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	78	103	371	82	102	410	801	-	-	860	-	-
Mov Cap-2 Maneuver	78	103	-	82	102	-	-	-	-	-	-	-
Stage 1	357	378	-	394	410	-	-	-	-	-	-	-
Stage 2	372	409	-	352	374	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s/v45.94			36.39		0.07		0.07	
HCM LOS	E		E					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	801	-	-	78	371	155	860	-	-
HCM Lane V/C Ratio	0.007	-	-	0.231	0.029	0.267	0.007	-	-
HCM Control Delay (s/veh)	9.5	-	-	64.2	15	36.4	9.2	-	-
HCM Lane LOS	A	-	-	F	B	E	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.8	0.1	1	0	-	-

Intersection												
Int Delay, s/veh	2.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕		↕	↗		↕	↗	
Traffic Vol, veh/h	29	0	11	2	1	10	14	836	18	19	871	21
Future Vol, veh/h	29	0	11	2	1	10	14	836	18	19	871	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	6	3	0
Mvmt Flow	30	0	11	2	1	10	14	862	19	20	898	22

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1839	1857	909	1837	1859	871	920	0	0	880	0	0
Stage 1	948	948	-	900	900	-	-	-	-	-	-	-
Stage 2	891	909	-	937	959	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	59	74	336	59	74	353	751	-	-	751	-	-
Stage 1	316	342	-	336	360	-	-	-	-	-	-	-
Stage 2	340	356	-	320	338	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	54	71	336	54	71	353	751	-	-	751	-	-
Mov Cap-2 Maneuver	54	71	-	54	71	-	-	-	-	-	-	-
Stage 1	308	333	-	329	353	-	-	-	-	-	-	-
Stage 2	322	350	-	301	329	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s/v102.9		28.87	0.16	0.21
HCM LOS	F	D		

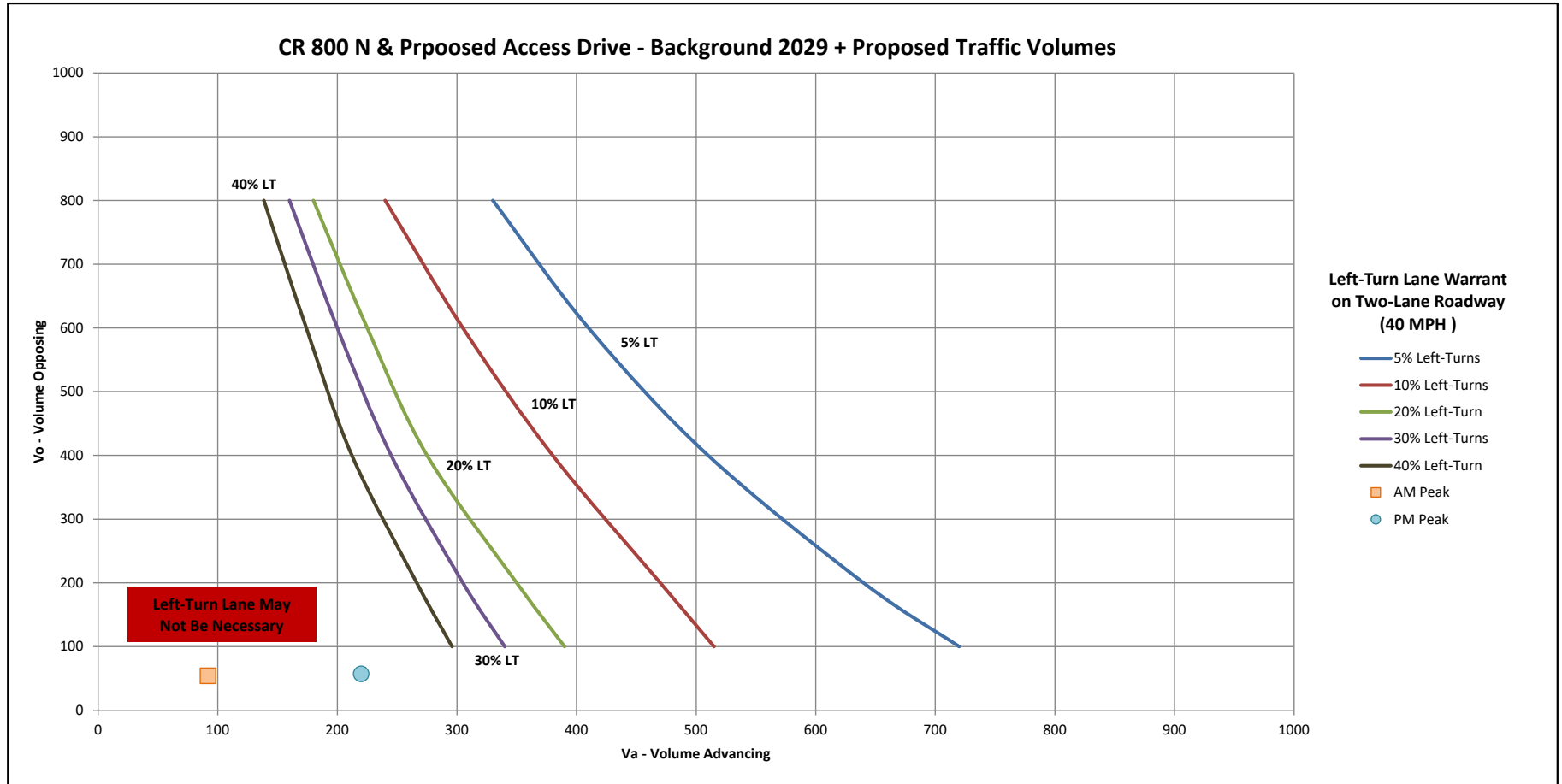
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	751	-	-	54	336	164	751	-	-
HCM Lane V/C Ratio	0.019	-	-	0.557	0.034	0.082	0.026	-	-
HCM Control Delay (s/veh)	9.9	-	-	135.8	16.1	28.9	9.9	-	-
HCM Lane LOS	A	-	-	F	C	D	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	2.2	0.1	0.3	0.1	-	-

***CR 800 N & PROPOSED DEVELOPMENT
ACCESS DRIVE***

***TURN LANE ANALYSIS
CAPACITY ANALYSIS***

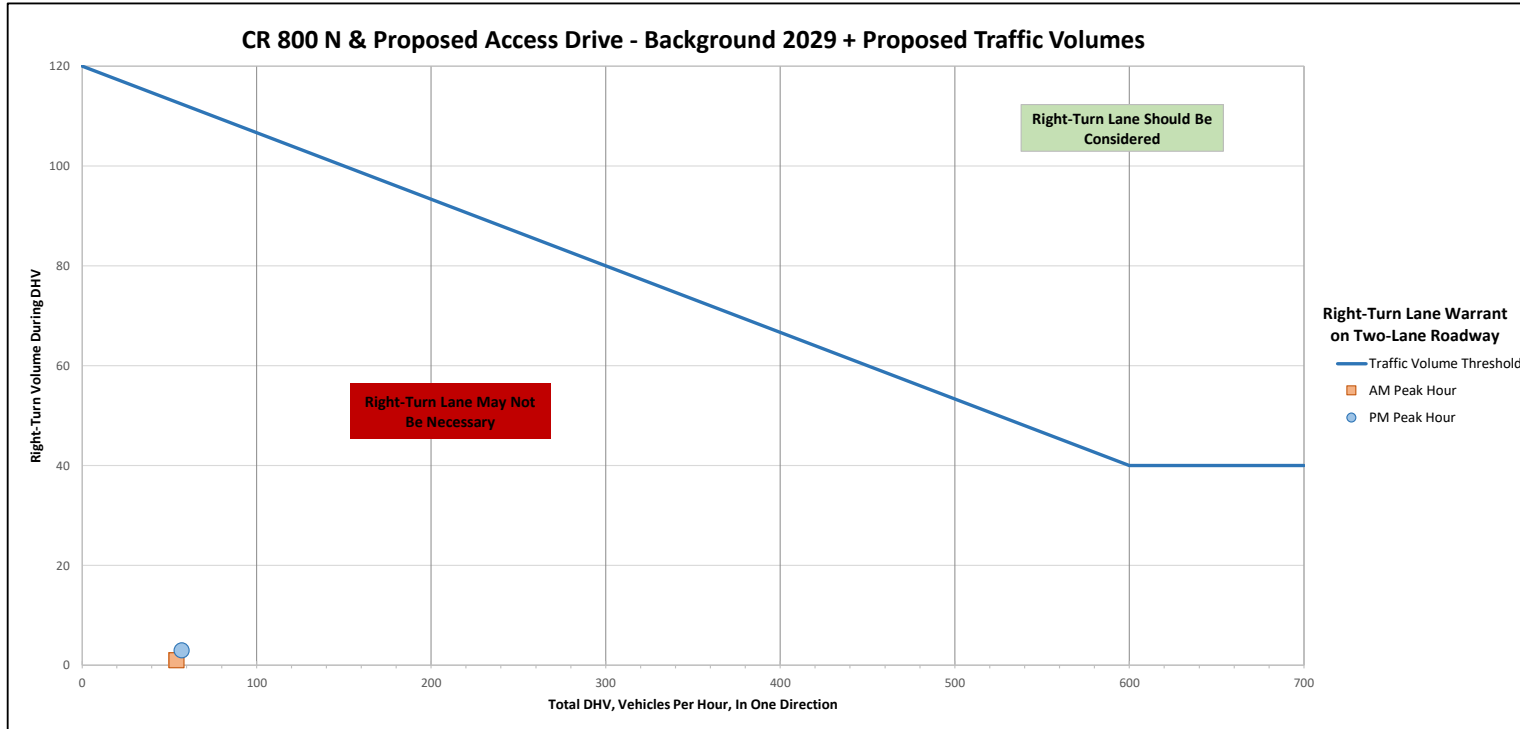
Operating Speed (mph)	Opposing Volume (veh/h)	Advancing Volume (veh/h)							
		5% Left Turns	10% Left Turns	15% Left Turns	20% Left Turns	25% Left Turns	30% Left Turns	35% Left Turns	40% Left Turns
40	800	330	240	207	180	168	160	146	139
	600	410	305	260	225	211	200	184	174
	400	510	380	320	275	258	245	224	212
	200	640	470	401	350	324	305	282	266
	100	720	515	446	390	360	340	313	296

AM Peak Hour Traffic Volume Input		PM Peak Hour Traffic Volume Input	
Advancing Volume (Va)	92	Advancing Volume (Va)	220
Opposing Volume (Vo)	54	Opposing Volume (Vo)	57
Left-Turn Volume	16	Left-Turn Volume	52
% Left-Turn	17%	% Left-Turn	24%
WARRANTED?	NO	WARRANTED?	NO



Total Approach Volume	Right-Turn Volume
0	120
600	40
700	40

AM Peak Hour Traffic Volume Input		PM Peak Hour Traffic Volume Input	
Total Approach Volume	54	Total Approach Volume	57
Right-Turn Volume	1	Right-Turn Volume	3
WARRANTED?	NO	WARRANTED?	NO



NOTE : For highways with a design speed below 80 km/h (50 mph) with a DHV < 300 and where right-turns > 40, an adjustment should be used. To read the vertical axis of the chart, subtract 20

Intersection						
Int Delay, s/veh	2.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	53	1	16	76	3	46
Future Vol, veh/h	53	1	16	76	3	46
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	58	1	17	83	3	50

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	59	0	176 58
Stage 1	-	-	-	-	58 -
Stage 2	-	-	-	-	117 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1545	-	814 1008
Stage 1	-	-	-	-	964 -
Stage 2	-	-	-	-	908 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1545	-	805 1008
Mov Cap-2 Maneuver	-	-	-	-	805 -
Stage 1	-	-	-	-	964 -
Stage 2	-	-	-	-	897 -

Approach	EB	WB	NB
HCM Control Delay, s/v	0	1.28	8.83
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	992	-	-	313	-
HCM Lane V/C Ratio	0.054	-	-	0.011	-
HCM Control Delay (s/veh)	8.8	-	-	7.4	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0.2	-	-	0	-

Intersection						
Int Delay, s/veh	2.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	54	3	52	168	2	30
Future Vol, veh/h	54	3	52	168	2	30
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	59	3	57	183	2	33

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	62	0	356 60
Stage 1	-	-	-	-	60 -
Stage 2	-	-	-	-	296 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1541	-	642 1005
Stage 1	-	-	-	-	962 -
Stage 2	-	-	-	-	755 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1541	-	616 1005
Mov Cap-2 Maneuver	-	-	-	-	616 -
Stage 1	-	-	-	-	962 -
Stage 2	-	-	-	-	724 -

Approach	EB	WB	NB
HCM Control Delay, s/v	0	1.76	8.86
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	967	-	-	425	-
HCM Lane V/C Ratio	0.036	-	-	0.037	-
HCM Control Delay (s/veh)	8.9	-	-	7.4	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0.1	-	-	0.1	-